

BACKGROUND – PURPOSE OF PROJECT

Lower Paxton Township and Lower Paxton Township Authority have made major financial and legal commitments to eliminating sanitary sewer overflows in the Beaver Creek drainage basin and reducing hydraulic overloads in the Swatara Township Joint Use Interceptor, as outlined in the Second Consent Decree of May 2002 entered into by Lower Paxton Township, Lower Paxton Township Authority, Swatara Township Authority, Pennsylvania Department of Environmental Protection, and private development interests.

The commitments made by Lower Paxton, and agreed to by the other signers of the Second Consent Decree, include the construction of the Beaver Creek Wet Weather Wastewater Treatment Plant (WWTP), the upgrade of the existing Beaver Creek Pump Station, and construction of major improvements to the wastewater conveyance system. These three construction activities are all part of the Township's plans to eliminate sanitary sewer overflows and to treat wastewater flows in excess of the Township's allocation in the Swatara Township Authority's Joint Use Interceptor.

Key portions of the Second Consent Decree are included in Appendix A. Photographs of the project area and drawings are included as part of this mitigation plan.

2. DESCRIPTION OF PROJECT

As discussed above, Lower Paxton has committed to the construction of the Beaver Creek Wet Weather Wastewater Treatment Plant (WWTP), the upgrade of the existing Beaver Creek Pump Station, and construction of major improvements to the wastewater conveyance system. Each of these items is discussed below.

Beaver Creek Wet Weather Wastewater Treatment Plant

The treatment plant site is located on the east side of Beaver Creek, east of Nyes Road in South Hanover Township, and is within the 100-year flood plain of Beaver Creek. The area where the proposed treatment plant will be constructed consists of an existing gravel drive, open grass-covered areas and underbrush. A 60-foot diameter water storage tank had been located on the site. The tank was demolished approximately 15 years ago when the adjacent water treatment plant was demolished. After the tank was demolished, the site returned to natural vegetation with the exception of the sanitary sewer right-of-way, sewage metering chamber and the drive.

In order to meet PA DEP's flood protection requirements for wastewater treatment facilities contained in Section 41.3 of PA DEP's *Domestic Wastewater Facilities Manual*, engineered fill will be used to raise the elevation of the treatment plant site and the access road to insure that the WWTP structures and electrical and mechanical equipment are protected from damage by the 100-year flood.

A detailed hydrologic and hydraulic analysis of the impacts of these site changes is included as part of the Joint Application for Pennsylvania Water Obstruction and Encroachment Permit and U.S. Army Corps of Engineers Section 404 Permit.

Beaver Creek Pump Station

The Beaver Creek Pump Station, located in the 100-year flood plain of Beaver Creek, was originally constructed in the early 1970s as part of township's wastewater conveyance system. In addition to

equipment replacement inside the pump station building, the project includes the replacement of the emergency generator which will be located outside the pump station building in the 100-year flood plain.

Conveyance System Improvements

Lower Paxton's Beaver Creek wastewater conveyance system was installed, for the most part, in the 1970s. As documented in the Second Consent Decree, the sewer system is subject to excessive amounts of inflow and infiltration (I/I) during wet periods. During periods of extended wet weather, the sanitary sewer system surcharges and wastewater overflows from various manholes with the wastewater eventually flowing into Beaver Creek and wetlands near the manhole overflow chambers.

The wastewater conveyance system improvements that will be constructed as part of Lower Paxton's commitment to meeting the terms of the Second Consent Decree include the installation of approximately 22,640 feet of gravity sewer and 8,300 feet of force main. As shown on the drawings included as part of this Mitigation Plan, most of the sewer will be replacement sewer located at, or very close to, where the sanitary sewer currently is located. Construction of these improvements to the conveyance system will necessitate 35 stream crossings with temporary road crossings, stream bank restoration at all stream crossings and at one additional area where the sewer is parallel to the stream, and 24 wetland crossings.

The Mitigation Plan for Lower Paxton's Beaver Creek Wastewater Treatment Facility, Beaver creek Pump station Improvements and Beaver creek Conveyance Improvements was developed in accordance with DEP and COE guidances and includes avoidance, mitigation, and compensation activities.

As discussed previously, the Beaver Creek wastewater conveyance system is an existing conveyance system, most of which was constructed 30 years ago. The sanitary sewers convey wastewater to Swatara Township Authority's Joint Use Interceptor; from here the wastewater is carried to the Swatara Township Authority's wastewater treatment plant where it is treated prior to discharge to the Swatara Creek. For the most part, Lower Paxton's sanitary sewer system was built following the drainage basins in the township with the conveyance system located adjacent to the streams (for example, in the eastern part of the Township, Beaver Creek, Nyes run and tributaries to those streams).

3. ACTIONS TAKEN TO AVOID AND MINIMIZE IMPACTS BY LIMITING THE DEGREE OR MAGNITUDE OF THE ACTION AND ITS IMPLEMENTATION

Preliminary Planning Activities

A detailed Alternatives Analysis to address wastewater needs within the Township was conducted as part of negotiations of the Second Consent Decree and Lower Paxton Township's Act 537 planning process. More than ten alternatives were considered with four alternatives selected for detailed evaluation, (1) a wet weather treatment facility located on Beaver Creek, (2) a conventional wastewater facility on Beaver Creek, (3) expansion of the Swatara Township WWTP and (4) replacement of the entire Beaver Creek sewer system. Of these, the selected alternative was improvements to the conveyance system to accommodate peak flows (the proposed improvements to the Beaver Creek Pump Station and Nyes Road and Beaver Creek Interceptors), construction of a peak flow/wet weather treatment plant (the proposed Beaver Creek Wet Weather WWTP), and on-going sewer system maintenance to keep inflow and infiltration at or below levels present when the Act 537 Plan Update was developed. Information from this Alternatives Analysis was included as part of the Joint Application for Pennsylvania Water Obstruction and Encroachment Permit and U.S. Army Corps of Engineers Section 404 Permit application.

With respect to the location of the Wet Weather WWTP, several sites were considered, including a site remote from the interceptor. Practicality and cost-effectiveness, however, constrain the alternatives to

sites that: (1) are adjacent to or in immediate proximity to the Nyes Run interceptor; (2) are downstream of sub-basins BC-1 – BC-10 of the Beaver Creek drainage basin to allow control of peak flows within the Swatara WWTP peak flow limit; and (3) provide access to Beaver Creek for adequate stream discharge. The primary reasons for selecting the proposed site include the following:

- Meets all three site criteria.
 - Continues similar land use (former water treatment plant site water storage facilities)
 - Wooded area provides some screening of facilities
 - Land available for sale
 - Least cost

DEP approved this Act 537 Plan Update on March 20, 2003. The proposed Beaver Creek Wet Weather WWTP and the improvements to the Beaver Creek Pump Station and the conveyance system are in accordance with the approved Act 537 Plan Update.

Activities taken during the project design to avoid and minimize impacts

Wastewater Treatment Plant

Photographs of the treatment plant site and adjacent area are included in Appendix B. The site is located on the east side of Beaver Creek. Nyes Road (S.R. 2019) parallels the stream on the west side. The old water works dam shown in the photographs will remain.

The following actions were taken during the design of the WWTP to avoid and minimize impacts to wetlands and to Beaver Creek and its associated floodway and flood plain.

- As one of the preliminary design activities, wetlands were delineated on the site. The treatment facilities were then designed to avoid impacting the wetlands to the greatest extent possible. These avoidance activities included:
 - The use of a retaining wall rather than the construction of a berm at one location along the eastern side of the treatment plant. Use of a retaining wall, the more expensive construction option, will minimize the impact to adjacent wetlands. This design change was made following the COE/DEP site visit in March 2004.
 - No construction activities in the wetlands that transverse the site except for a small area near one of the new trickling filters.

The facilities located in the flood plain occupy as small an area as possible in accordance with good engineering practices and in accordance with South Hanover Township requirements.

The facilities have been located as far as possible from Beaver Creek and still minimizing impacts to the wetlands that traverse the site.

The new driveway will be located where the existing drive is located.

No construction activities will take place within 10 feet of the top of the stream bank, thus protecting the stream bank and not impacting shading of the stream that the trees and brush provide.

- The Treatment Unit structure, which is located in the 100-year flood plain, utilizes common wall construction to minimize the foot print of the structure. The structure contains the sedimentation

basins, ultraviolet disinfection units, electrical area, work area, and office and utility space. If this structure had been designed as separate stand-alone units, as is normally done, a greater area would have been needed.

The Screen structure, which is located in the 100-year flood plain, has been designed to also contain the chemical feed facilities needed for the removal of phosphorus from the wastewater. As with the Treatment Unit structure, one structure will serve more than one purpose, reducing the amount of construction in the 100-year flood plain.

Ultraviolet disinfection will be used rather than chlorination. This will prevent the discharge of chlorinated wastewater to Beaver Creek and reduce the area required for chlorine contact tanks.

The treatment plant site will be provided with public water supplied by United Water Company. Rather than open-cut Beaver Creek to install the waterline, the waterline will be installed using directional drilling. This will minimize temporary construction impacts to Beaver Creek.

- The amount of paving is the minimum needed for the treatment facility and the width of the driveway is the minimum acceptable to South Hanover Township, as this is required by ordinance.

Beaver Creek Pump Station

Photographs of the pump station site are included in Appendix B. The pump station, an existing building constructed in the 1970s, is located in the 100-year flood plain of Beaver Creek. Rather than constructing a replacement pump station, the existing pump station will be rehabilitated thus minimizing impacts to the floodplain. Except for the replacement emergency generator and replacement fence, the modifications to the pump station will be conducted inside the building. Temporary wastewater pumping facilities will be used while the interior of the pump station is modified. Decisions made during the design of the project to minimize impacts to the flood plain included design of the new meter pit and surge relief pit to be flush with the ground surface and, therefore, not impact flood flows at the site, and removal of the existing hydrogen peroxide storage tank and foundation located in the flood plain.

Conveyance System

Stream Crossings

As discussed previously, the project calls for replacement of an existing gravity interceptor sewer and includes several stream crossings. Crossings are dictated by stream location, wetlands, surrounding topography and locations of existing collection sewer connections. As every effort was made to limit the amount of stream disturbance, the number of stream crossings for this project is considered the minimum necessary within these physical constraints.

Table 1 provides information on the stream crossing that are part of the project including the name of the stream, stream width, whether the crossing will be at or near an existing crossing or if the crossing will be a new crossing and information on how the stream bank will be rehabilitated. Approximately 400 feet of the 31,000 feet of sewer to be constructed will cross under perennial and intermittent streams. Except for one crossing where stream velocities are high during rain events, the stream banks will be revegetated with native plantings such as willow, redbud and elderberry.

Table 1
Stream Crossings

Crossing No.	Drawing No.	Stream	Municipality	Location	Stream Width (ft.)	Type Crossing	Bank Rehab.	Discussion
1	2	Beaver Ck.	Lower Paxton South Hanover	approx. 720 ft. S of intersection of Nyes Rd. & Willoughby Rd.	32	1	1	Sewer must cross Beaver Creek which is perpendicular to the sewer route. There is no alternative.
2	2	Nyes Run	Lower Paxton	approx. 510 ft. S of intersection of Nyes Rd. & Willoughby Rd.	14	3	2	An additional stream crossing to that required for existing sewer, this is necessary in order to avoid the bridge that was constructed after the sewer was originally installed. Disruption to the stream is considered to be less than that associated with excavating the existing sewer in the steep embankment next to the bridge.
3	3	Nyes Run	Lower Paxton	approx. 275 ft. S of intersection of Nyes Rd. & Willoughby Rd.	10	2	2	Existing interceptor is being replaced in place. There is no alternative to crossing the creek.
4	3	Nyes Run	Lower Paxton	approx. 460 ft. W of intersection of Nyes Rd. & Willoughby Rd.	15	3 (note a)	2	Interceptor re-routed to avoid a long stretch of wetlands over the existing sewer. Also, the existing stream crossing south of new manhole B16A will be stabilized and existing manhole B16 will be removed from Nyes Run.
5	4	Nyes Run	Lower Paxton	approx. 1,500 ft. W of intersection of Nyes Rd. & Willoughby Rd.	22	2	3	Interceptor re-routed adjacent to existing interceptor. Although there is no alternative to the stream crossing, the new location is more perpendicular to the stream and reducing disturbance to the stream bank.
6	4	Nyes Run	Lower Paxton	approx. 2,200 ft. NW of intersection of Nyes Rd. & Willoughby Rd.	13	2	2	Interceptor immediately adjacent to existing interceptor. There is no alternative to stream crossing as there is no room for construction of the sewer any further north because of steep slopes to the creek.

Table 1
Stream Crossings

Crossing No.	Drawing No.	Stream	Municipality	Location	Stream Width (ft.)	Type Crossing	Bank Rehab.	Discussion
7	5	UNT Nyes Run	Lower Paxton	approx. 900 ft. SE of intersection of Old Union Deposit Rd. & Nyes Rd.	5	2	4	Interceptor crosses a drainage swale tributary to Nyes Run at the location of existing crossing. Crossing cannot be avoided. Interceptor location has been re-located to the east of the existing interceptor from MH B25A to MH B29A (at crossing) to reduce disturbance to Nyes Run banks and to move out of forested wetlands.
8	7	UNT Nyes Run	Lower Paxton	approx. 1,900 ft. SE of intersection of Locust Ln. & Nyes Rd.	5	2	4	Force main crossing of drainage swale tributary to Nyes Run adjacent to location of existing crossing. The crossing cannot be avoided
9	7	UNT Nyes Run	Lower Paxton	approx. 1,380 ft. SE of intersection of Locust Ln. & Nyes Rd.	5	2	4	Force main crossing of intermittent stream tributary to Nyes Run adjacent to location of existing crossing. The crossing cannot be avoided
10	7	UNT Nyes Run	Lower Paxton	approx. 1,120 ft. SE of intersection of Locust Ln. & Nyes Rd.	5	2	4	Force main crossing of drainage swale tributary to Nyes Run at location of existing crossing. The crossing cannot be avoided.
11	7	UNT Nyes Run	Lower Paxton	approx. 960 ft. SE of intersection of Locust Ln. & Nyes Rd.	5	2	4	Force main crossing of drainage swale tributary to Nyes Run adjacent to location of existing crossing. The crossing cannot be avoided
12	8	UNT Nyes Run	Lower Paxton	approx. 600 ft. N of intersection of Locust Ln. & Nyes Rd.	5	2	2	Force main crossing of unnamed tributary to Nyes Run adjacent to location of existing crossing. The crossing cannot be avoided
13	9	UNT to Nyes Run	Lower Paxton	approx. 75 ft. SE of intersection of Nyes Rd. & Red Top Rd.	8	3 (note b)	2	Force main crossing of unnamed tributary to Nyes Run. The crossing is necessary to avoid multiple stream crossings and substantial stream bank disturbance that would be required if the force main followed the existing interceptor right-of-way on the west side of the creek.

Table 1
Stream Crossings

Crossing No.	Drawing No.	Stream	Municipality	Location	Stream Width (ft.)	Type Crossing	Bank Rehab.	Discussion
14	9	UNT to UNT to Nyes Run	Lower Paxton	approx. 175 ft. NE of intersection of Nyes Rd. & Red Top Rd.	5	3	4	Force main crossing of unnamed tributary to unnamed tributary to Nyes Run. The crossing is necessary to avoid multiple stream crossings and substantial stream bank disturbance that would be required if the force main followed the existing interceptor right-of-way on the west side of the creek
15	9	UNT to Nyes Run	Lower Paxton	approx. 700 ft. NE of intersection of Nyes Rd. & Red Top Rd.	10	1	2	Force main crossing of unnamed tributary to Nyes Run. The crossing is necessary to avoid multiple stream crossings and substantial stream bank disturbance that would be required if the force main followed the existing interceptor right-of-way on the west side of the creek.
16	9	UNT to UNT to Nyes Run	Lower Paxton	approx. 950 ft. NE of intersection of Nyes Rd. & Red Top Rd.	5	1	4	Force main crossing a drainage swale tributary to an unnamed tributary to Nyes Run adjacent to location of existing sewer. There is no practical alternative.
17	10	UNT to Nyes Run	Lower Paxton	approx. 350 ft. S of intersection of Nyes Rd. & Hunters Run Rd.	13	3 (note c)	2	Force main crossing an unnamed tributary to Nyes Run. Given site constraints (location of Nyes Road, steep banks, closeness of Nyes Road to stream), there is no practical alternative.
18	10	UNT to Nyes Run	Lower Paxton	approx. 200 ft. S of intersection of Nyes Rd. & Hunters Run Rd.	15	3 (note c)	2	Force main crossing an unnamed tributary to Nyes Run. Given site constraints (location of Nyes Road, steep banks, closeness of Nyes Road to stream), there is no practical alternative.
19	10	UNT to Nyes Run	Lower Paxton	approx. 90 ft. S of intersection of Nyes Rd. & Hunters Run Rd.	10	1	2	Force main crossing an unnamed tributary to Nyes Run. Given site constraints (location of Nyes Road, steep banks, closeness of Nyes Road to stream), there is no practical alternative.

Table 1
Stream Crossings

Crossing No.	Drawing No.	Stream	Municipality	Location	Stream Width (ft.)	Type Crossing	Bank Rehab.	Discussion
20	12	UNT to Beaver Creek	Lower Paxton	approx. 1,000 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	5	3	2	Force main crossing an unnamed intermittent stream tributary to Beaver Creek. There is no practical alternative
21	14	Beaver Creek	Lower Paxton West Hanover	approx. 100 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	28	2	3	Interceptor sewer constructed adjacent to existing interceptor at existing stream crossing. Given the upstream topography (steep slope stream embankment), there is no practical alternative.
22	15	Beaver Creek	Lower Paxton West Hanover	approx. 1,900 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	15	2	3	Interceptor replacing existing interceptor in place. The location is necessary to accumulate future connections.
23	15	UNT to Beaver Creek	Lower Paxton	approx. 2,050 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	7	2	2	Interceptor crossing an unnamed intermittent stream tributary to Beaver Creek adjacent to existing interceptor crossing.
24	15	Beaver Creek	Lower Paxton West Hanover	approx. 2,750 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	40	2	3	Interceptor adjacent to location of existing interceptor. Stream crossing is necessary to avoid wetlands and steep embankment upstream on south side of the creek.
25	16	UNT to Beaver Creek	West Hanover	approx. 3,550 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	5	2	4	Interceptor crossing a drainage swale tributary to Beaver Creek at location of existing crossing. There is no practical alternative.
26	16	Beaver Creek	Lower Paxton West Hanover	approx. 3,900 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	25	2	3	Interceptor at location of existing interceptor. Stream crossing is necessary to connect to existing tributary collection sewer at MH B116A and interceptor crossing under Jonestown Road.
27	17	UNT to Beaver Creek	Lower Paxton	approx. 4,200 ft. N of intersection of Yorkshire Dr. & Deaven Rd.	5	2	4	Interceptor crossing a drainage swale tributary to Beaver Creek. There is no practical alternative.

Table 1
Stream Crossings

Crossing No.	Drawing No.	Stream	Municipality	Location	Stream Width (ft.)	Type Crossing	Bank Rehab.	Discussion
28	18	Beaver Creek	Lower Paxton West Hanover	approx. 2,120 ft SE of intersection of Blue Ridge Ave. & Chelton Ave.	20	2	3	Interceptor at location of existing interceptor. Stream crossing necessary to connect the existing sewer crossings of Jonestown Road and Rt. 81, which will both be left in place.
29	18	UNT to Beaver Creek	Lower Paxton	approx. 2,030 ft SE of intersection of Blue Ridge Ave. & Chelton Ave.	5	2	2	Interceptor crosses an unnamed tributary to Beaver Creek at location of existing interceptor. Stream crossing necessary to connect the existing crossings of Jonestown Road and Rt. 81, which will both be left in place. Also avoids disturbing steep embankment on west side of unnamed tributary near MH B126
30	18	UNT to Beaver Creek	Lower Paxton	approx. 1,360 ft SE of intersection of Blue Ridge Ave. & Chelton Ave.	5	2	2	Interceptor crosses unnamed tributary to Beaver Creek adjacent to location of existing interceptor from MH B127 along existing cleared path. Following existing location minimizes disturbing wooded areas around stream.
31	19	UNT to Beaver Creek	Lower Paxton	approx. 830 ft SE of intersection of Blue Ridge Ave. & Chelton Ave.	5	2	2	Crossing unnamed tributary of Beaver Creek near existing interceptor location. Relocation of the sewer between B128A and B130A actually eliminates two stream crossings and one embankment disturbance. Also, this crossing 31 is more perpendicular to the stream than existing, limiting bank disturbance.
32	19	UNT to Beaver Creek	Lower Paxton	approx. 400 ft SW of intersection of Blue Ridge Ave. & Chelton Ave.	5	2	2	Crossing unnamed tributary of Beaver Creek near existing interceptor location. Relocation of the sewer further south upstream of this crossing actually eliminates two stream crossings, one of them either in the stream or embankment for about 75 feet between existing manholes B135 and B116
33	20	UNT to Beaver Creek	Lower Paxton	approx. 750 ft SW of intersection of Blue Ridge Ave. & Chelton Ave.	10	2	2	Crossing unnamed tributary of Beaver Creek near existing interceptor. Crossing is necessary to avoid steep, wooded embankment on the south side of the creek

Table 1
Stream Crossings

Crossing No.	Drawing No.	Stream	Municipality	Location	Stream Width (ft.)	Type Crossing	Bank Rehab.	Discussion
34	20	UNT to Beaver Creek	Lower Paxton	approx. 1,140 ft W of intersection of Blue Ridge Ave. & Chelton Ave.	10	2	2	Interceptor follows close to existing interceptor,
35	20	UNT to Beaver Creek	Lower Paxton	approx. 1,260 ft W of intersection of Blue Ridge Ave. & Chelton Ave.	10	2	2	Crossing the tributary to Beaver Creek twice. Consideration was given to moving the sewer to the side of the creek to avoid these crossings, but is essentially kept in the existing location to provide an access point of connection for future development that may occur to the south, without the need for a stream crossing. Also, no additional right-of-way is needed, and disturbance to landscaped area and mature trees on the north side of the creek is avoided
Total feet of sewer to be placed under the stream bed					402			
Total feet of stream bank impacted					1,750			Estimated at 25 feet on each side of stream
Feet of stream bank to be re-vegetated with native plantings					1,400			Estimated at 10 feet on each side of sewer on both sides of stream
Feet of stream bank to be re-vegetated with grass					300			Estimated at 5 feet; overtop of sewer
Feet of stream bank with rock riprap bank protection without native plantings					50			Beaver Creek crossing

Crossing Types

1. Replacement crossing near existing crossing.
2. Replacement crossing at or in immediate vicinity of existing crossing.
3. New crossing.

Bank Rehabilitation Types

1. Rock rip-rap.
2. BioD-block 2000 with jute matting; native plantings.
3. BioD-block 2000 with jute matting and rock rip rap; native plantings.
4. Jute matting; native plantings.

Notes

- a. Crossing location moved to current location to reduce impact to wetlands.
- b. Crossing location moved from current location to reduce number of crossings and stream bank disturbance.
- c. Crossing location moved to avoid impacts to Nyes Road.

Temporary road crossings will be located immediately adjacent to the locations of the utility line stream crossings. These crossings will provide access for the construction vehicles to the work area. The following procedures will be used to ensure that the sanitary sewers are constructed to minimize the potential for future problems.

Sewer Watertight Integrity and Alignment: In order that the sewer crossing under a stream will be rendered watertight and remain in alignment, all pipeline for stream crossing will be mechanical joint, ductile iron pipe between the manholes on either side of the crossing, and installed in a trench at a sufficient depth below the stream to prevent impact by stream flow or changes in streambed. At crossings of some drainage swales, concrete encasement is used.

Sufficient Cover: Stream crossings on the project are believed to be of sufficient depth to meet DEP's requirement of one (1) foot of cover in rock and three (3) feet of cover in other materials.

Watertight Manholes: Watertight manholes will be installed where the sewer is near a stream. These manholes will prevent surface water from entering the system, helping to control I/I.

Wetlands

Several decisions were made during the design of the conveyance system improvements to minimize or avoid impacts to wetlands. These are summarized in Table 2.

Activities that will be taken during the construction the of the project to avoid and minimize impacts

All construction activities will follow the erosion and sedimentation control plan approved by the Dauphin County Conservation District. Key items in this plan include:

- Stream crossings will be made in the dry.
- Stream banks will be stabilized and re-vegetated; stabilization information is on drawing E&S-3.
- Stream bank stabilization will be done immediately following the completion of the utility line installation.
- Clay dams will be installed at both sides of the utility line stream crossings.
- Pads, mats or other similar devices will be used when crossing wetlands.
- Where the sewers are installed in wetlands, the topsoil in wetlands will be segregated from the subsoil and then be used to backfill the trench backfill. Backfill is to be machine tamped and compacted to limit vertical drainage.
- Clay dams will be installed at both ends of wetlands and every 100 feet through the wetlands.
- Construction width is limited to 30-feet in non-forested wetlands and 20 feet in forested wetlands.
- Disturbed wetlands will be restored to pre-construction elevations and contours.

4. ACTIONS TAKEN TO RECTIFY THE IMPACT BY REPAIRING, REHABILITATING OR RESTORING THE IMPACTED ENVIRONMENT

All areas disturbed during construction will be restored in accordance with DEP's chapter 102 requirements. All construction activities will be in accordance with the Erosion and Sedimentation Control Plans approved by the Dauphin County Conservation District.

At the pump station site, all temporary pumping facilities will be removed from the flood plain as will the existing hydrogen peroxide tank and foundation. The Authority will continue to maintain the pump

Table 2
Minimization and Avoidance Activities

Drawing No.	Minimization or avoidance activity
C-3	The original design for the section of sewer from new manhole (MH) B8A to Nyes Run located the replacement sewer adjacent to the existing sewer in wetlands. The design was revised, moving the location of the new sewer closer to Nyes Road, to eliminate the sewer crossing over 800 feet of wetlands and to reduce construction access in wetlands requirements.
C-3	Sewer location from new MHB14 to new MH B15 was moved to avoid wetlands.
C-4	Existing sewer from near existing MHB19 to near existing MHB21 is located very close to Nyes Run, crossing or adjacent to wetlands. The replacement sewer has been designed to be located away from the stream bank to protect the stream and to be located, where possible, at the edge of wetlands thus minimizing wetlands impacts.
C-4	From new MHB22-A to new MHB24A the sewer location was moved during design to the east so that the sewer could be located in an existing cleared path in the wetlands to avoid forested wetlands.
C-5	From new MH 26A to new MH 29A, rather than replace the sewer where it is currently located, during design the location was moved slightly to the east in an existing cleared path to avoid forested wetlands.
C-5/6	From existing MH 29 to existing MH 3 ^A , the decision was made during design to replace the sewer where it is currently located to avoid impacting forested wetlands.
C-6	At the old Union Deposit Road area the original design had construction access to the sewer located on both sides of old Union Deposit Road near the old Union Deposit Road bridge. During design, construction access on the north side of the road was moved to near the intersection of Nyes Road with old Union Deposit Road to avoid wetlands.
C-6	The new sewer will be constructed under old Union Deposit Road by boring rather than open-cut. The location of the boring receiving pit was moved during design to avoid wetlands.
C-6	Forested wetlands are present near the location of the replacement sewer from new MHB34A to new MHB35A. Although the replacement sewer is not located in wetlands, the original design included a small section of the wetlands in the construction easement. This area was removed during design from the construction easement.
C-14	From new MHB94A to new MHB98A the replacement sewer location was moved slightly during design to the west of the existing sewer to avoid forested wetlands.
C-15/16	During design the location of the replacement sewer from new MHB100A to new MH110A was moved slightly north of the existing sewer, which is located in forested wetlands, in an existing path to limit impacts to forested wetlands.

Table 2
Minimization and Avoidance Activities

Drawing No.	Minimization or avoidance activity
C-17	From new MHB113A to new MHB115A, during design the replacement sewer location was moved to an existing path to minimize impacts to wetlands. The existing sewer is located in wetlands.
C-19	The existing sewer from near new MH130A to near new MHB132A is located in wetlands. The replacement sewer location was moved during design to an existing path in the wetlands to minimize wetland impacts.
C-20	The existing sewer between existing MHB141 and existing MHB143 is located in forested wetlands. During design the replacement sewer location was moved north of the existing sewer to avoid forested wetlands.

station site as it currently does, mowing the area adjacent to the pump station building while not mowing or cutting the vegetation that grows on the stream bank.

At the locations of the conveyance system improvements, stream banks and wetlands disturbed during construction of the sewer will be stabilized as part of the construction activities. Specific stabilization procedures are contained on the project drawings.

5. ACTIONS TAKEN TO REDUCE OR ELIMINATE THE IMPACT OVER TIME BY PRESERVATION AND MAINTENANCE OPERATIONS DURING THE LIFE OF THE ACTION

At the treatment plant site, areas near the new treatment facilities will be maintained as mowed lawn. The areas adjacent to Beaver Creek will remain as they currently are, with native vegetation. The wetlands present at the site will also remain as they currently are. The operation of the WWTP will not impact the wetlands.

The pump station site will be maintained as is currently done. The lawn area will be mowed and the area adjacent to Beaver Creek will not be mowed and will remain in native vegetation.

Wetlands disturbed during the construction of the conveyance system improvements will be restored to pre-construction conditions after construction is completed. Trench plugs will be installed to prevent dewatering of the wetlands.

Stream banks disturbed during the construction of the conveyance system improvements will be restored during the construction process. Except for one location near the junction of Nyes Run and Beaver Creek, all bank restoration will include the use of native plantings.

6. PRIMARY AND SECONDARY IMPACTS

Primary impacts include permanent and temporary impacts to wetlands and stream banks. As shown on Table 3, permanent impacts to wetlands are estimated at only 0.045 acres for the entire project, including the wet weather treatment plant and 31,000 LF of pipe line. Of this, most is associated with a 1,490 sq.ft. (0.034 acres) area of wetlands identified on the treatment plant site. The net increase in permanent impact from the existing conditions due to the installation of some larger diameter manholes is 0.007 acres. As indicated on Table 3, temporary impacts are estimated at 6.1 acres.

As shown in Table 1, only 50 feet of the stream bank disturbed during construction will be stabilized with non-vegetative bank rehabilitation. Of the remaining 1,700 feet, 1,400 will be re-vegetated with native plantings while 300 feet (the area on top of the sewer trench) will be re-vegetated with grass. Because all stream banks will be re-habilitated, all stream bank impacts are considered temporary.

Secondary impacts include the temporary removal of trees at several of the wetland and stream crossing areas. As discussed above, the stream banks will be re-vegetated to re-establish the riparian corridor, to provide shading protection for the stream and help prevent future erosion.

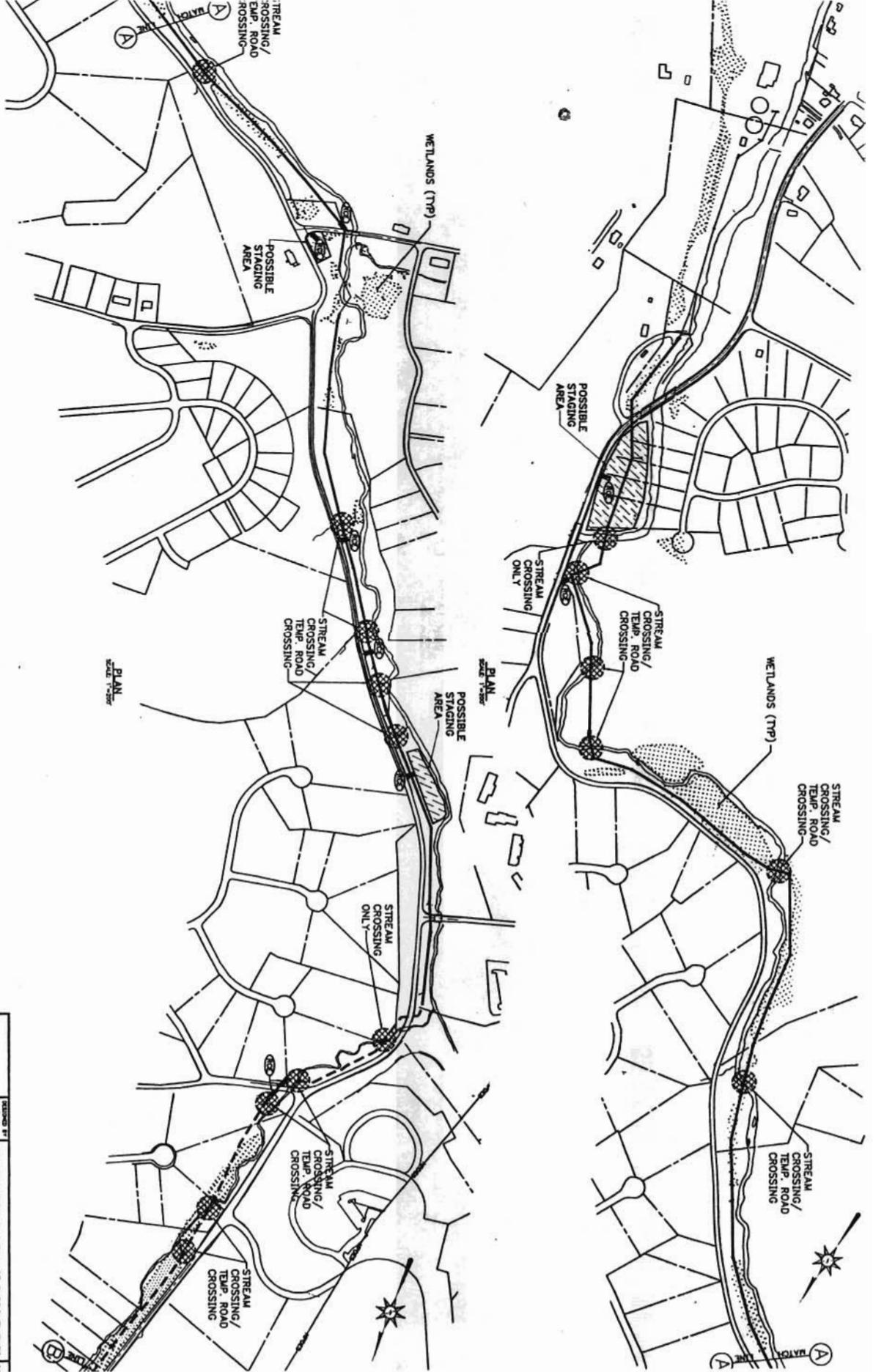
7. COMPENSATION FOR THE IMPACT OF THE PROJECT

As shown on Table 3, the project will result new permanent impacts to 0.041 acres of wetlands. As compensation for this impact, Lower Paxton Township Authority proposes to improve conditions along the stream banks of Nyes Runs at several locations. Erosion of the Nyes Run stream banks has become a

significant problem over the years. In some areas, the stream channel has shifted to such an extent that some existing manholes are now exposed in the stream channel. This further destabilizes the stream. The stabilization of the stream channel impacts not only the immediate area, but reduces the sediment loading to all areas downstream, improving water quality. The proposed stream improvement projects are as follows:

Remove existing manhole B16 from Nyes Run and install approximately 10 linear feet of stream bank protection at this area to minimize further bank erosion. This manhole and adjacent area is shown on Photo MH-1.

2. Install approximately 40 linear feet of stream bank protection along Nyes Run south of Stream Crossing 14 (see stream crossing photo) where the stream is eroding the bank.
3. Remove existing interceptor manhole B50 from an unnamed tributary to Nyes Run as shown on in photograph MH-3 and re-vegetate the stream bank.
4. Install approximately 150 feet of stream bank protection near Stream Crossing No. 33 (see stream crossing photo) replacing the existing deteriorating bank protection.

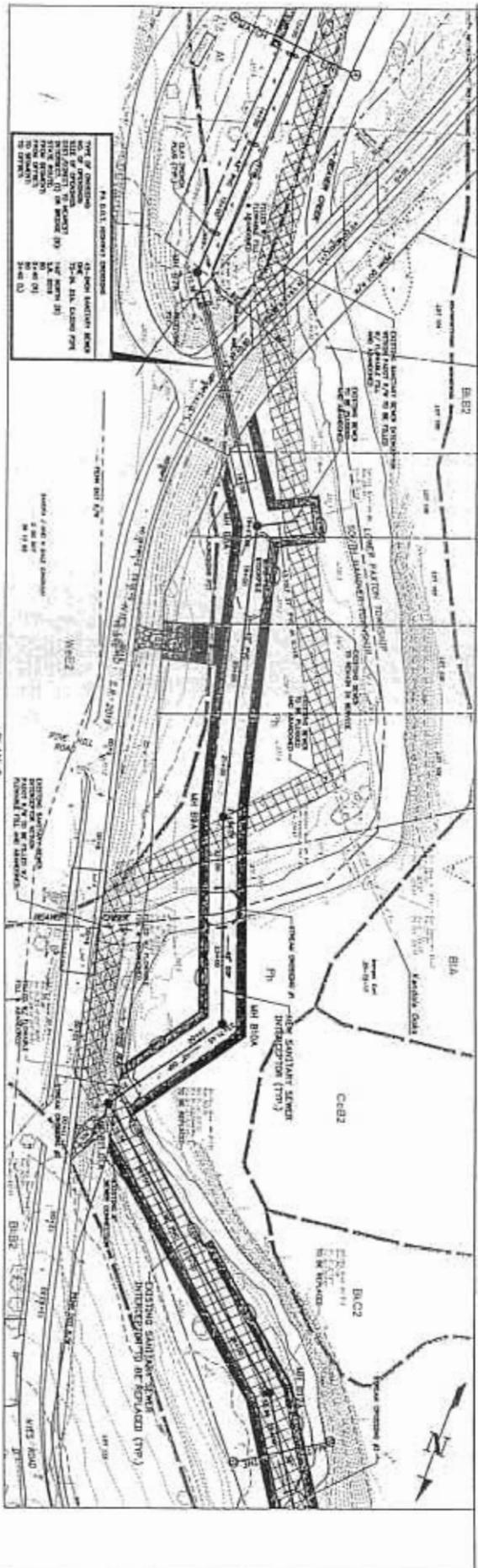


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19	11-1-21	CEP	REVISION TO PERMITTING
20	11-1-21	CEP	REVISION TO PERMITTING

BEAVER CREEK CONVEYANCE IMPROVEMENTS

OWNER: LORAIN VALLEY TOWNSHIP AUTHORITY
 DESIGNER: LORAIN VALLEY SOUTH WARDEN AND WEST WARDEN TOWNSHIPS
 CONSULTANT: PERKINS+WILKINSON
KEY PLAN 1

CEP Engineering Services
 1111 1st Street, West, Suite 100, Grand Forks, ND 58701
 Phone: 701.775.1111 Fax: 701.775.1112
 www.perkinswilkinson.com

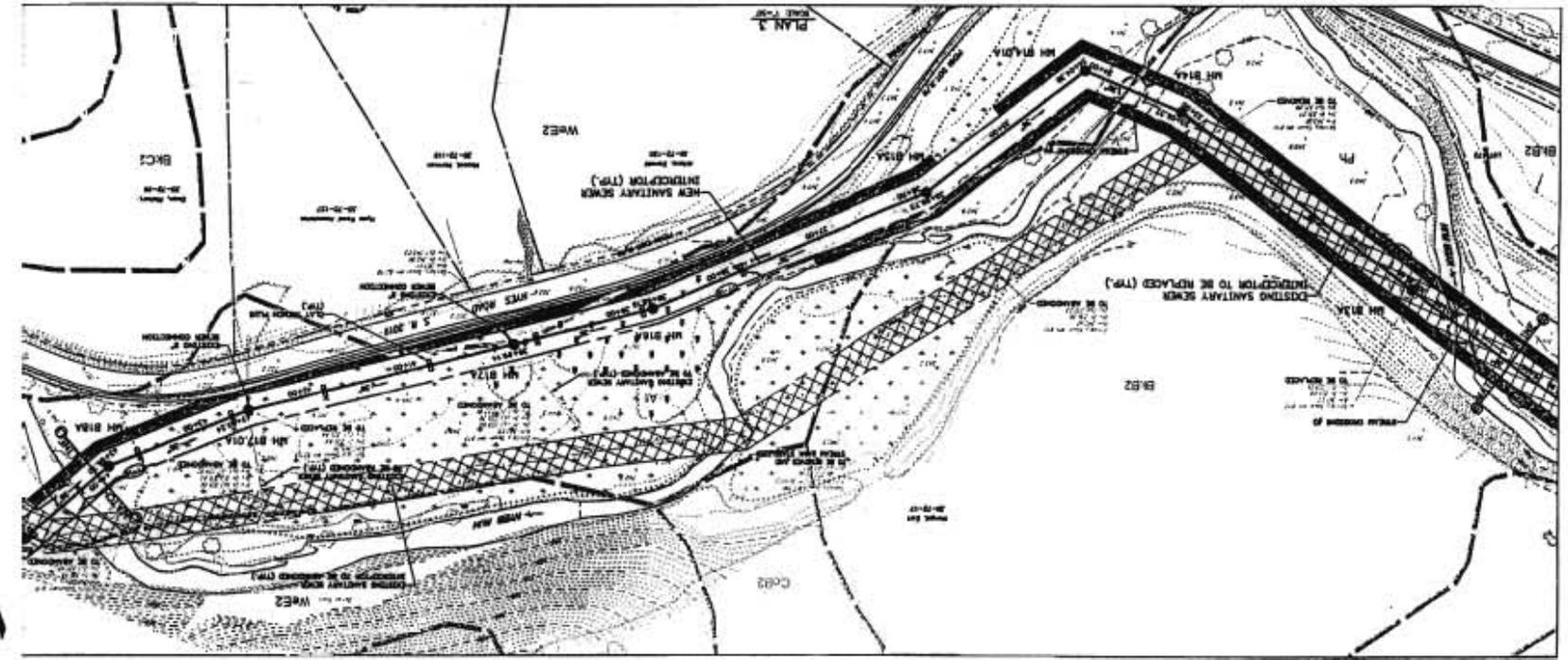
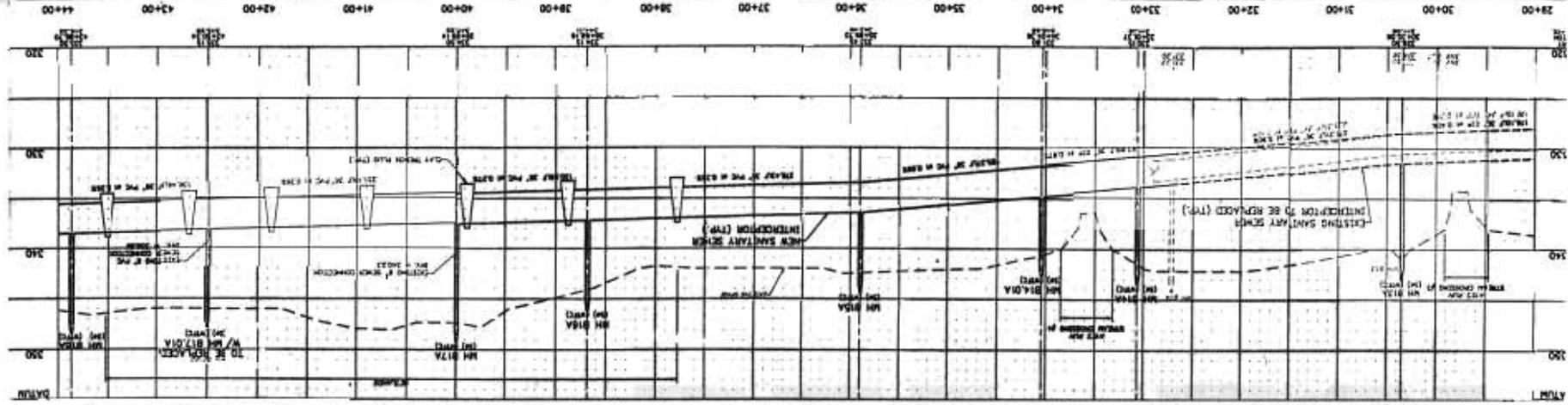


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BY	CEI
CHECKED BY	CEI
APPROVED BY	CEI
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CLIENT	LOUISIANA STATE UNIVERSITY
LOCATION	LOUISIANA STATE UNIVERSITY
SCALE	PLAN / PROFILE 2
DATE	1-1-59
BY	CEI
CHECKED BY	CEI
APPROVED BY	CEI
PROJECT NO.	1-1-59
PROJECT NAME	BEAVER CREEK CONVEYANCE IMPROVEMENTS
CLIENT	LOUISIANA STATE UNIVERSITY
LOCATION	LOUISIANA STATE UNIVERSITY
SCALE	PLAN / PROFILE 2
DATE	1-1-59
BY	CEI
CHECKED BY	CEI
APPROVED BY	CEI

BEAVER CREEK CONVEYANCE IMPROVEMENTS
 LINER FACTOR THROUGH AUTHORITY
 LONCK PATTON ENGINEERS
 3400 W. 13TH AVENUE
 DENVER, CO 80202
 TEL: 303.733.1100
 FAX: 303.733.1101
 C-3

NO.	DESCRIPTION	DATE
1	PRELIMINARY PLAN	11-15-01
2	REVISION: ADDITIONAL NOTES	11-15-01
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4	REVISION: ADDITIONAL NOTES	11-15-01
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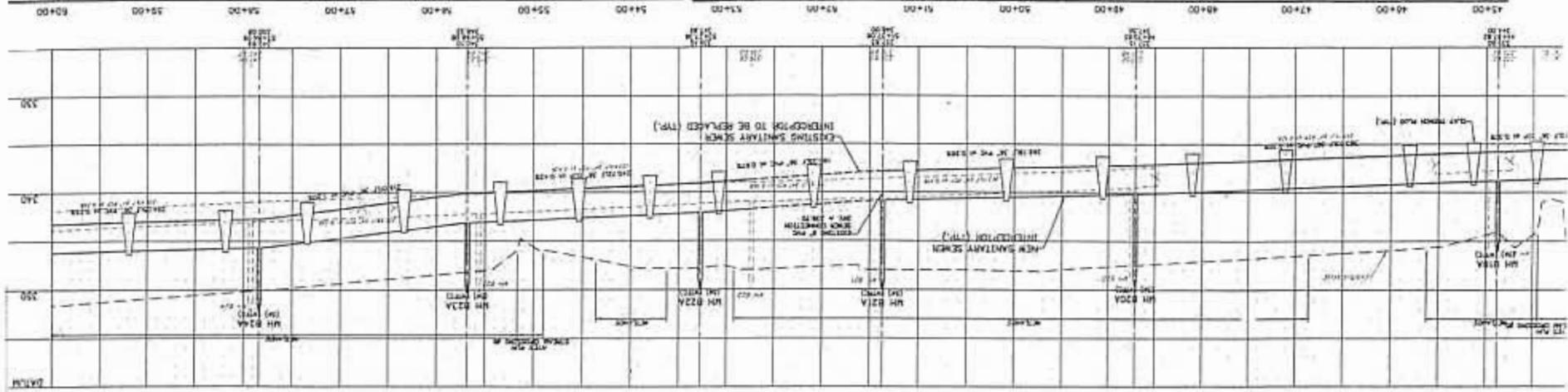
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 VERTICAL 1"=4'



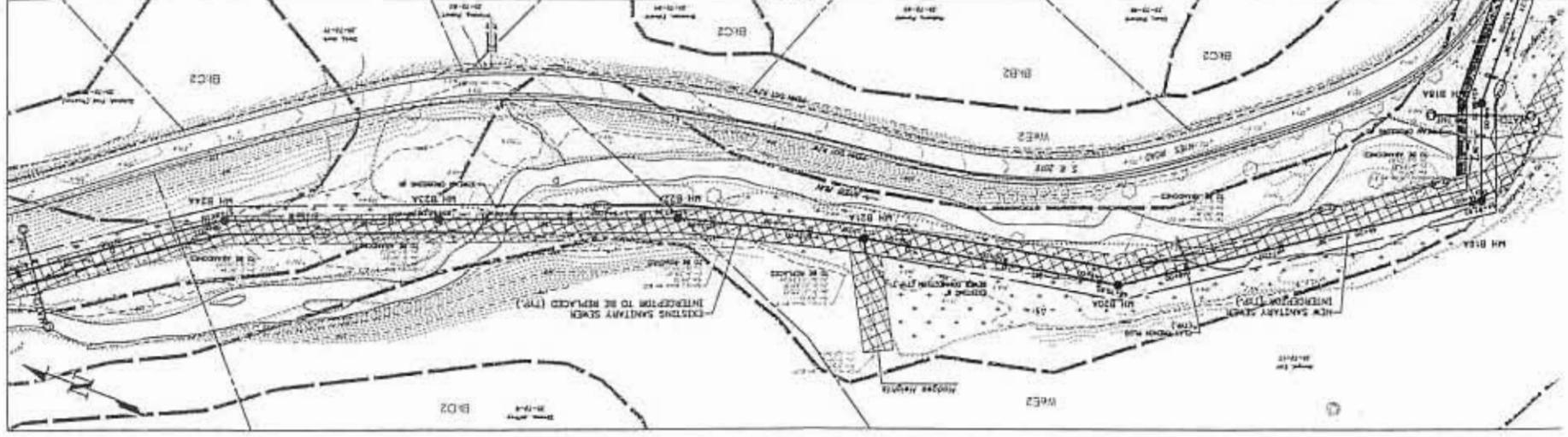
PROFILE
SCALE: 1" = 10'

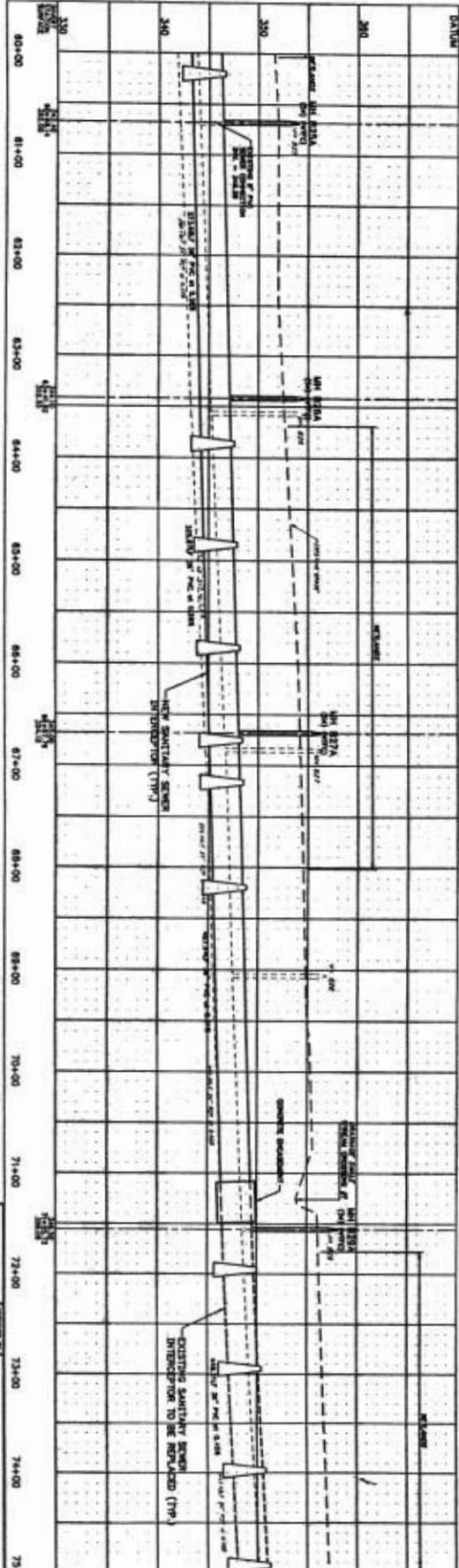
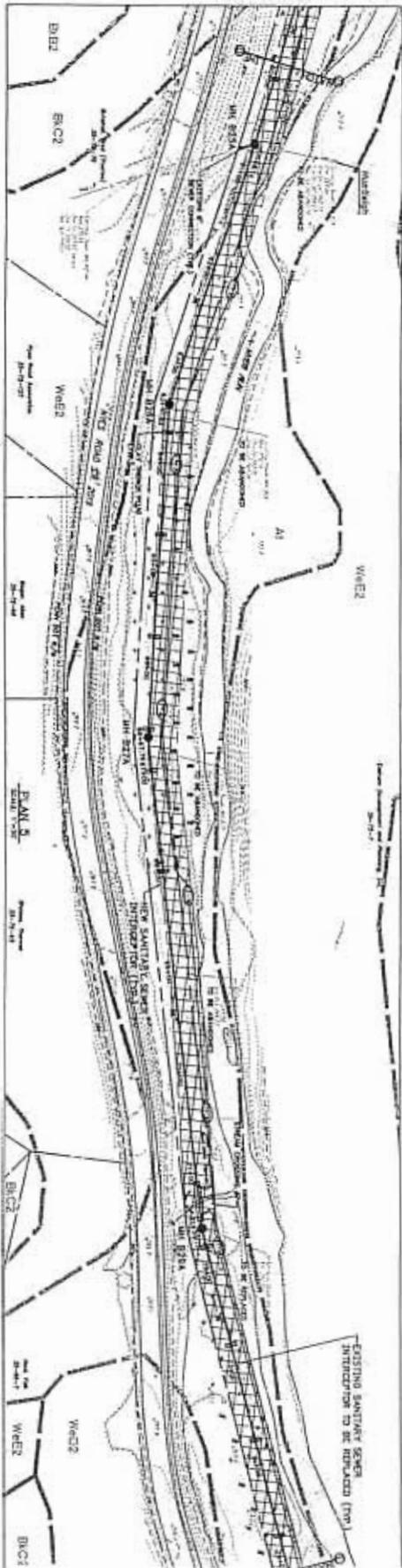
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PLAN 4
SCALE: 1" = 10'





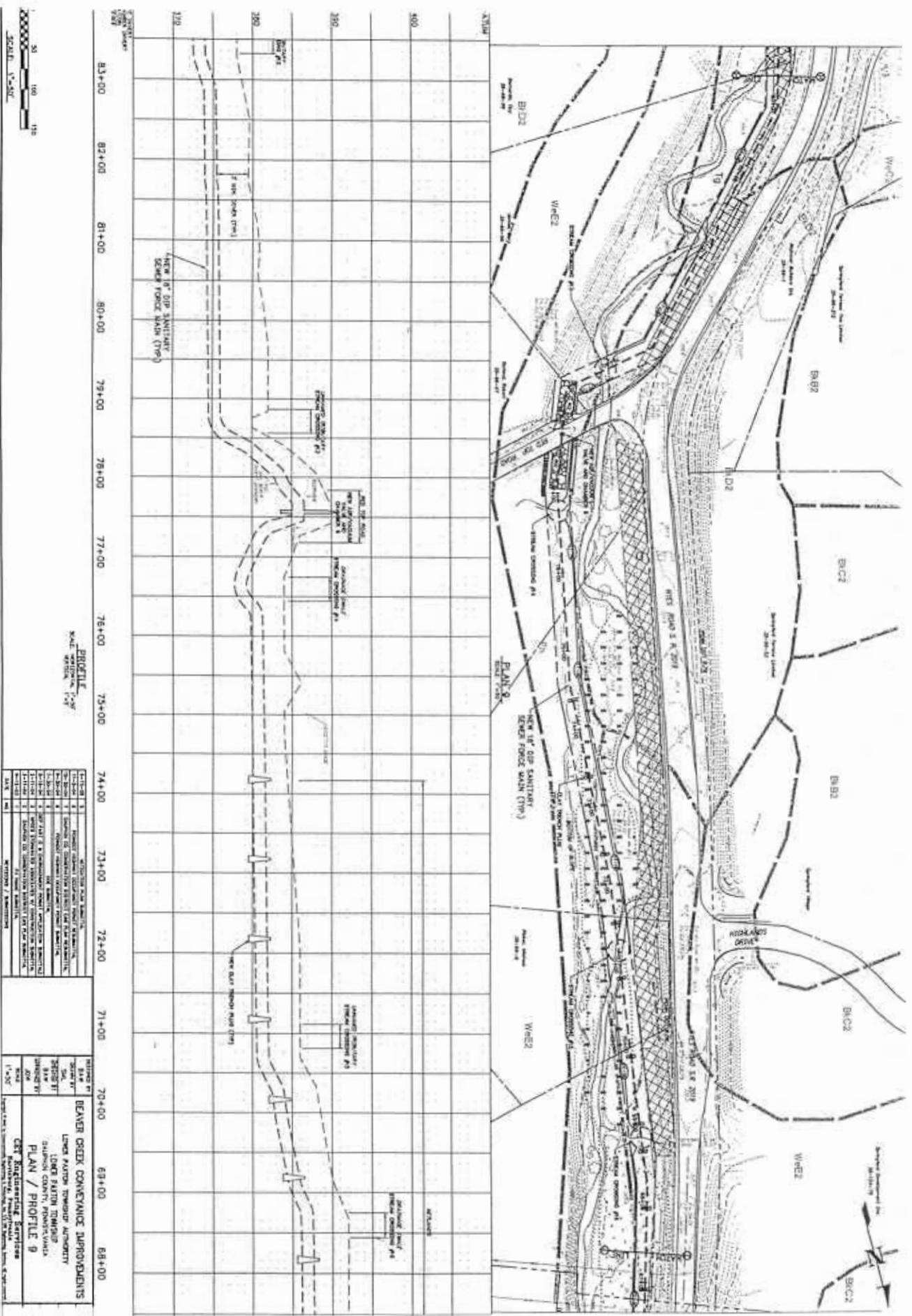
SCALE: 1"=50'

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DESIGNED BY: BEAVER CREEK CONFORMANCE IMPROVE
 DRAWN BY: LINDA HAYDON TOWNSHIP AUTHORITY
 CHECKED BY: LINDA HAYDON TOWNSHIP AUTHORITY
 PLAN / PROFILE 5
 CBT Engineering Services
 1100 S. 10th Street, Suite 100
 Lincoln, NE 68502
 (402) 441-1100

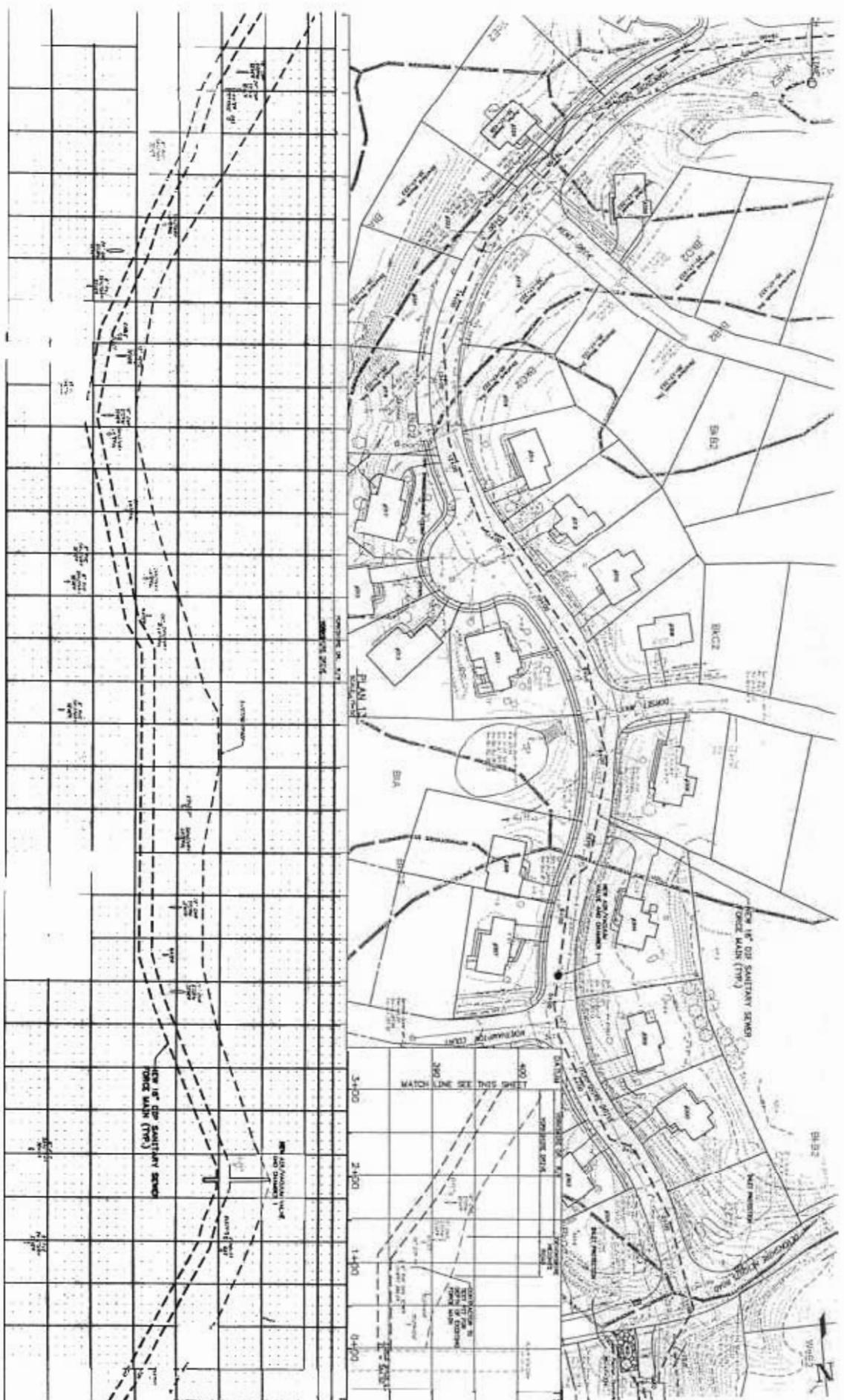


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BEAVER CREEK CONVEYANCE IMPROVEMENTS
 LINCOLN COUNTY TOWNSHIP
 SALMON COUNTY, OREGON
 PLAN / PROFILE 9
 C&E Engineering Services
 1100 NE Oregon Street, Suite 200
 Medford, Oregon 97504
 Phone: (541) 753-1100
 Fax: (541) 753-1101
 Website: www.candee.com





19+00 18+00 17+00 16+00 15+00 14+00 13+00 12+00 11+00 10+00 9+00 8+00 7+00 6+00 5+00

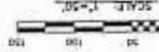
PROFILE
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PLAN 13

SCALE: 1"=50'

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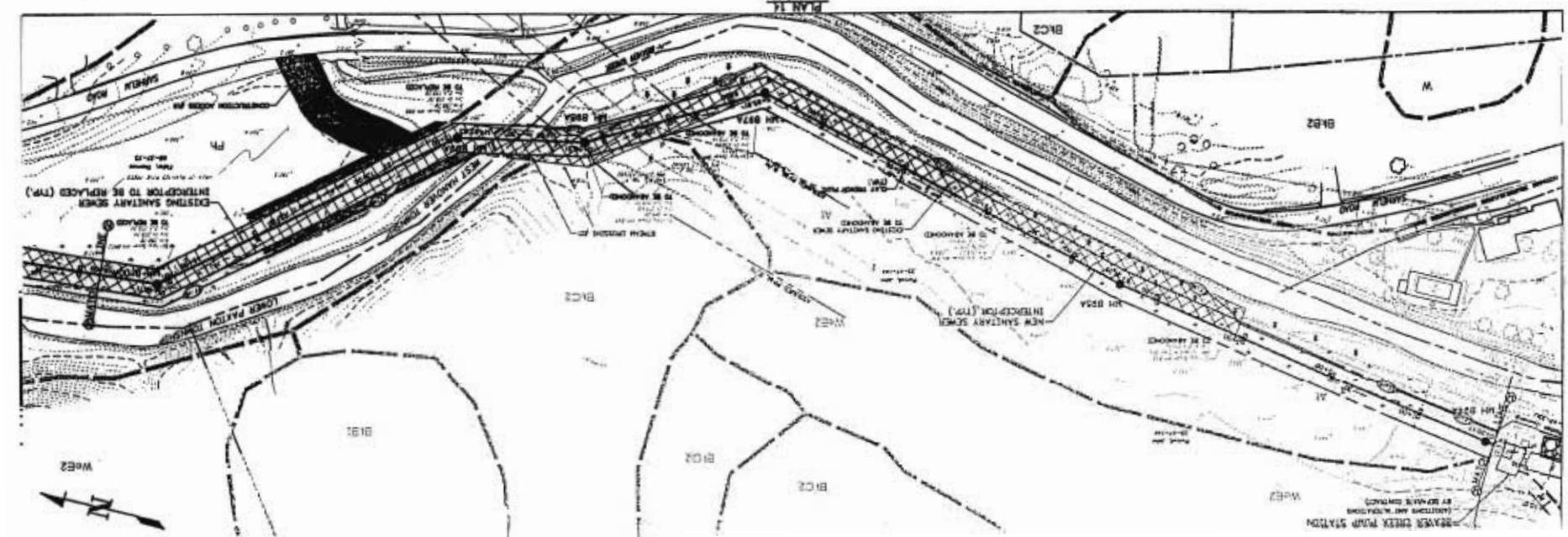
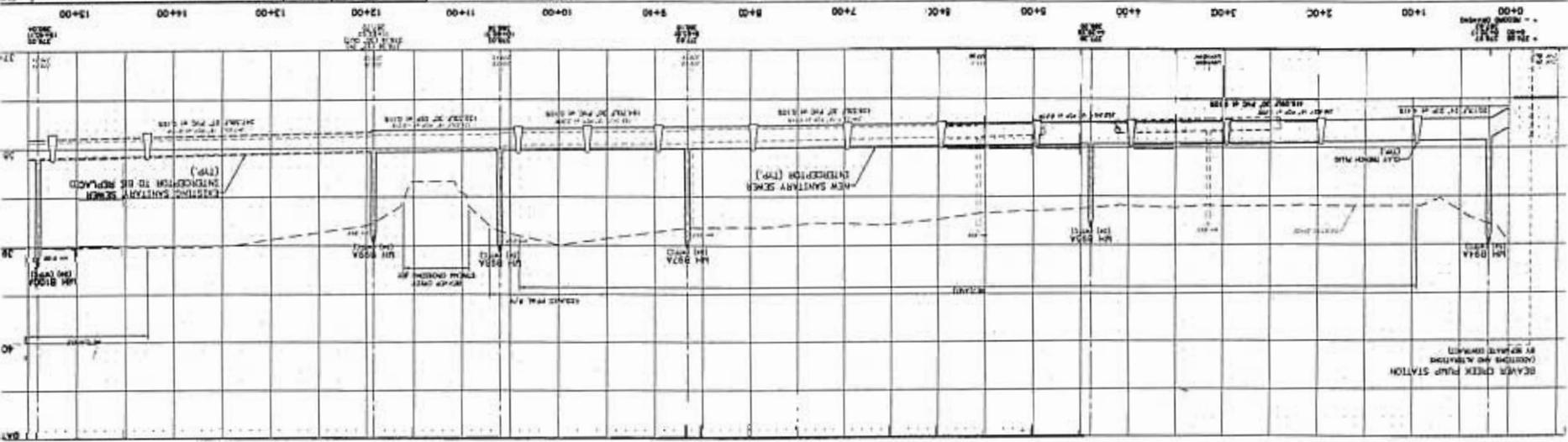
BEAVER CREEK CONVEYANCE IMPROVEMENTS
LITCHFIELD TOWNSHIP AUTHORITY
DANIEL KALTON TOWNSHIP
DIVISION OF PUBLIC UTILITIES
PLAN / PROFILE 13
CITY ENGINEERING SERVICES
11/11/03



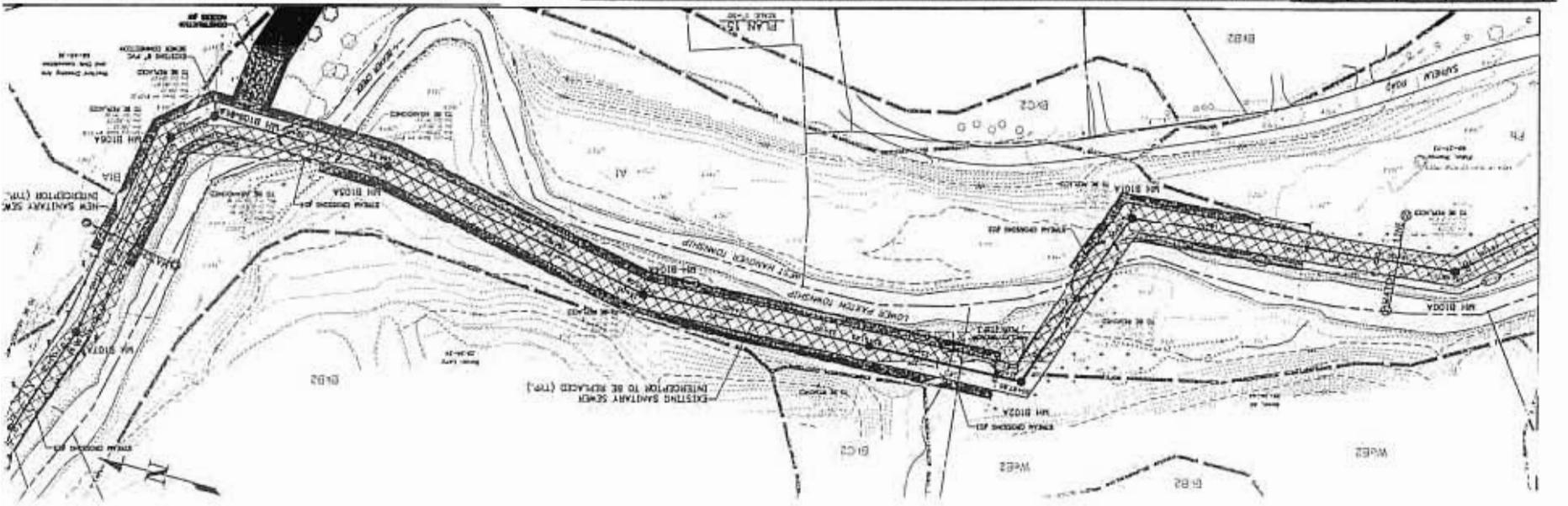
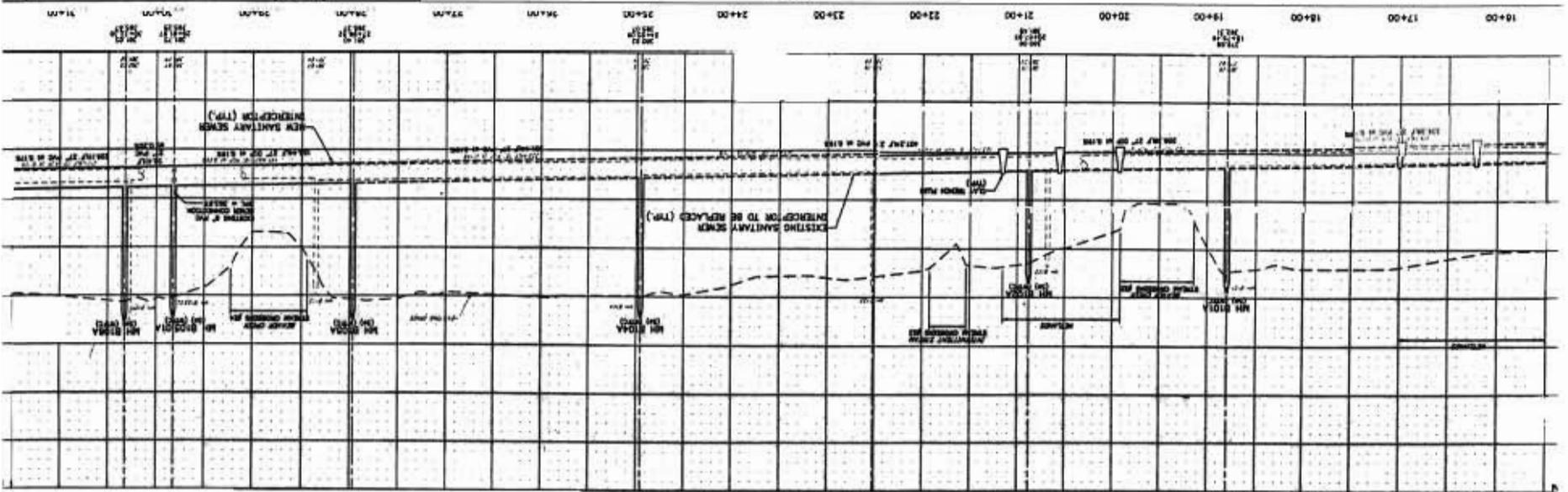
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8	ISSUED FOR PERMITS	1-11-00
9	ISSUED FOR PERMITS	1-11-00
10	ISSUED FOR PERMITS	1-11-00

PROJECT
 BEAVER CREEK
 LOWER FAYTON TOWNSHIP

BEAVER CREEK CONVEYANCE IMPROVEMENTS
 LOWER FAYTON TOWNSHIP AUTHORITY
 LOWER FAYTON TOWNSHIP
 DALLAS COUNTY, TEXAS
 PLAN / PROFILE 14
 C&E Engineering Services
 1500 West Loop South, Suite 100
 Houston, Texas 77027



BEAVER CREEK CONVEYANCE IMPROVEMENTS LOWER PATTON TOWNSHIP JOHNSON COUNTY, PENNSYLVANIA PLAN / PROFILE 15 CMT Engineering Services 1-2024		DATE: 1-2024 DRAWN BY: [Name] CHECKED BY: [Name] SCALE: 1"=50' PROJECT: BEAVER CREEK CONVEYANCE IMPROVEMENTS
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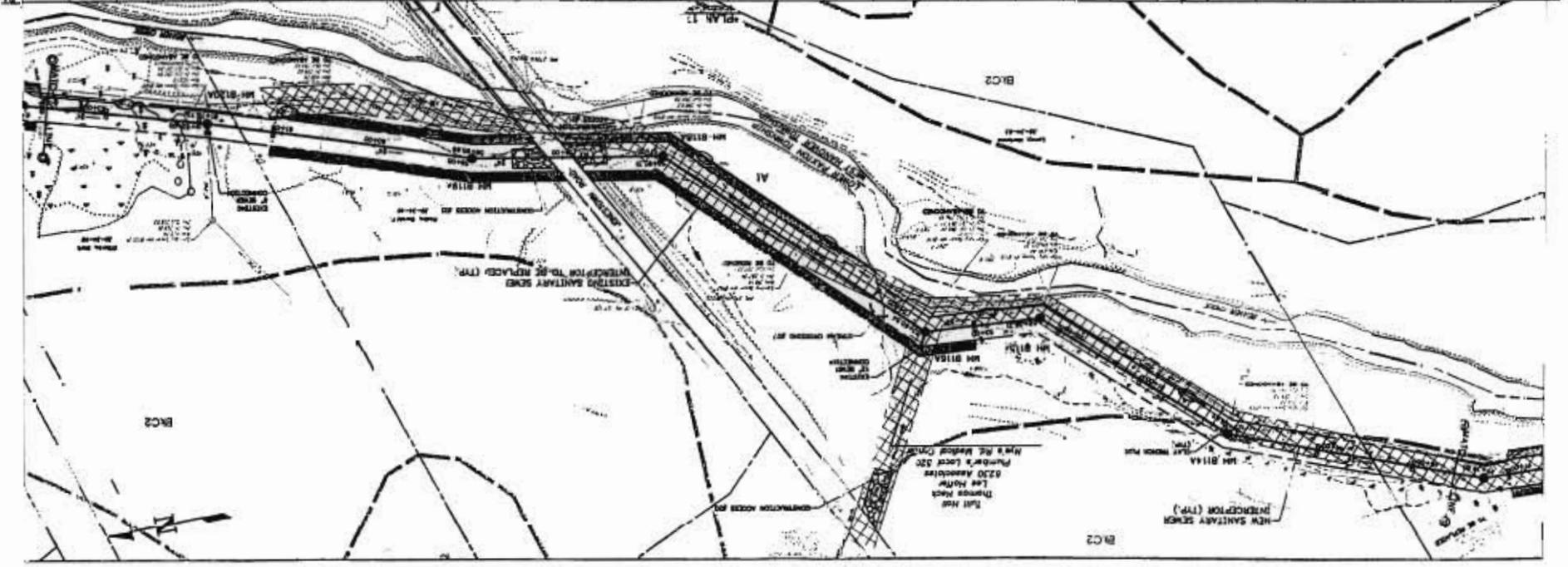
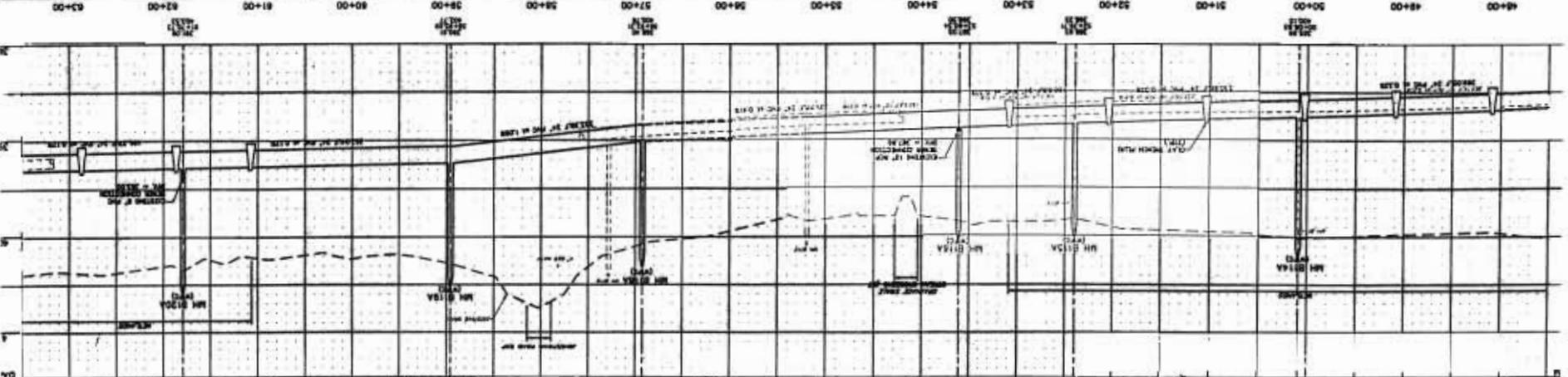


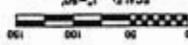


PROFILE
SCALE HORIZONTAL 1"=50'

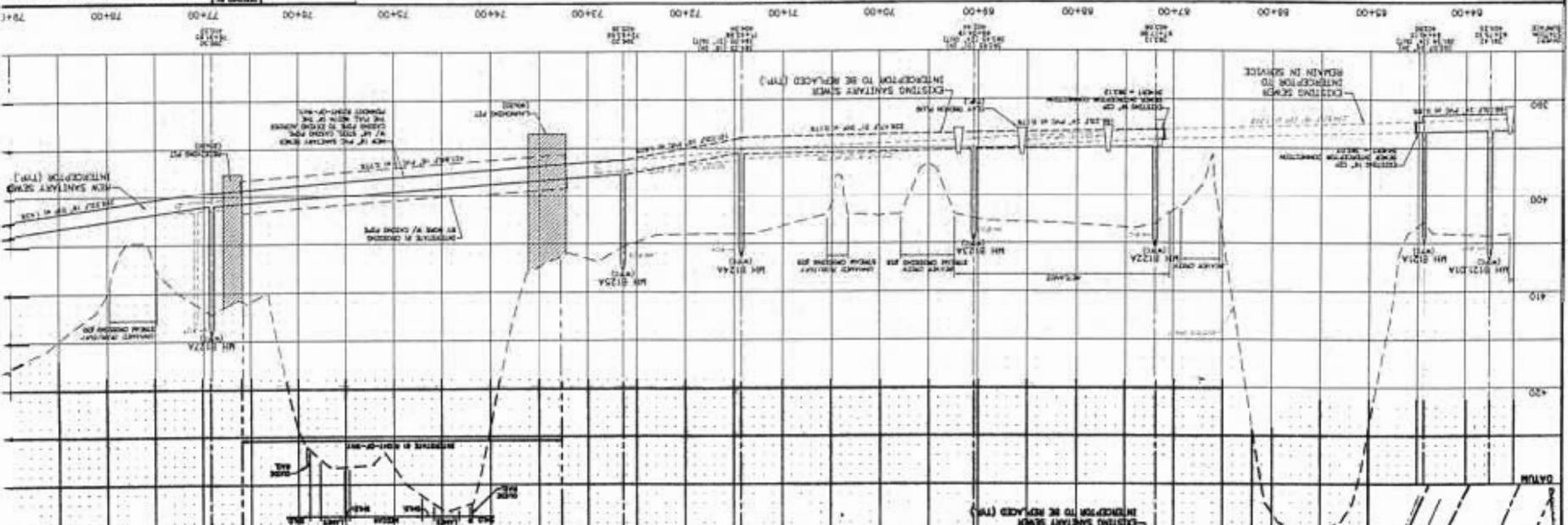
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50	ADJUSTMENT FOR GRADE

BEAVER CREEK CONVEYANCE IMPROVEMENTS
LOWER FAULTON TOWNSHIP
GARMIN COUNTY, PENNSYLVANIA
PLAN / PROFILE 17
CET Engineering Services
17-00



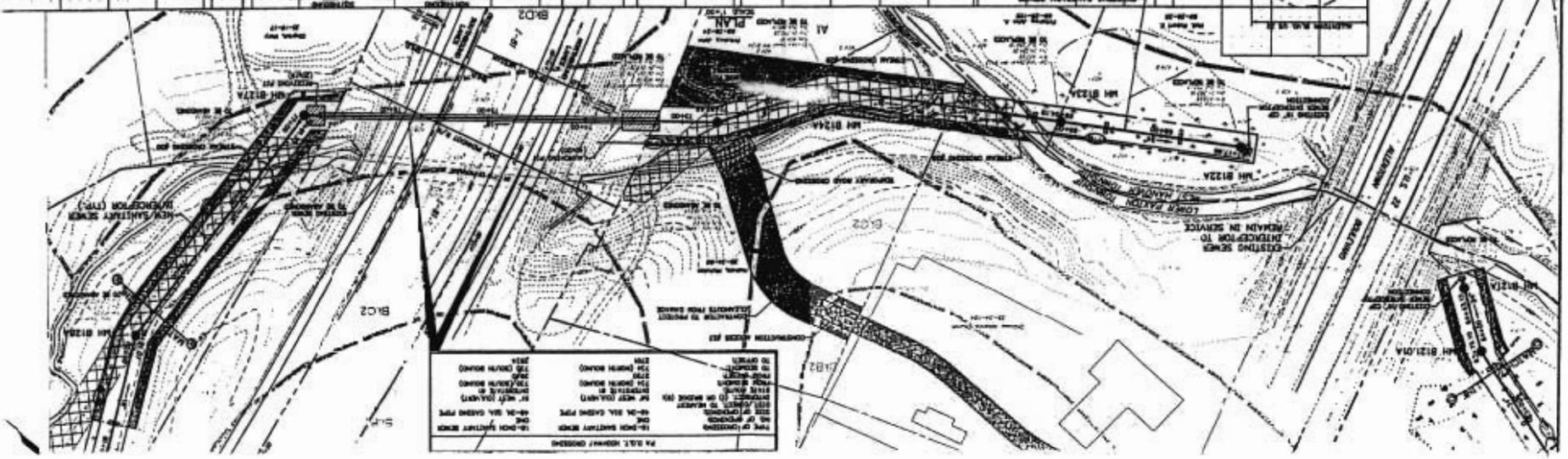


PROFILE
SCALE HORIZONTAL 1"=50'

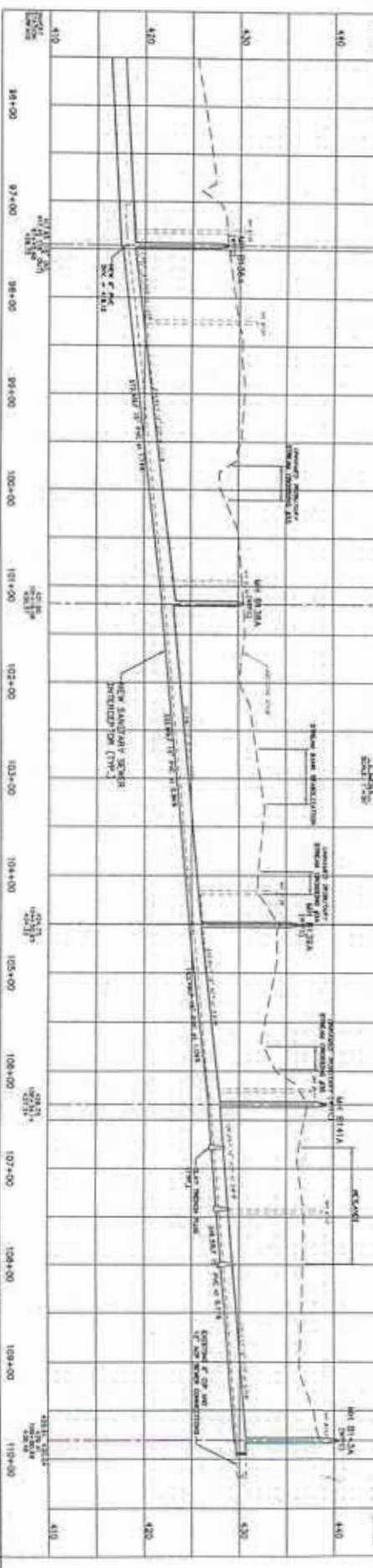


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4	REVISED TO SHOW CONSTRUCTION DETAILS	2-15-12
5	REVISED TO SHOW CONSTRUCTION DETAILS	3-15-12
6	REVISED TO SHOW CONSTRUCTION DETAILS	4-15-12
7	REVISED TO SHOW CONSTRUCTION DETAILS	5-15-12
8	REVISED TO SHOW CONSTRUCTION DETAILS	6-15-12
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49	REVISED TO SHOW CONSTRUCTION DETAILS	11-15-15
50	REVISED TO SHOW CONSTRUCTION DETAILS	12-15-15

BEAVER CREEK CONVEYANCE IMPROVE
 DRAWN BY: J. J. ...
 CHECKED BY: J. J. ...
 DATE: 11-15-11
 SHEET NO. 18 OF 18
 PROJECT NO. 11-001



SYMBOL	DESCRIPTION
(Symbol)	NEW SANITARY SEWER
(Symbol)	EXISTING SANITARY SEWER
(Symbol)	INTERCEPTION TO BE REPLACED (TR)
(Symbol)	REMAIN IN SERVICE
(Symbol)	MANHOLE
(Symbol)	CONCRETE MANHOLE
(Symbol)	CAST IRON MANHOLE
(Symbol)	STEEL MANHOLE
(Symbol)	WOOD MANHOLE
(Symbol)	CONCRETE BOX CULVERT
(Symbol)	CAST IRON BOX CULVERT
(Symbol)	STEEL BOX CULVERT
(Symbol)	WOOD BOX CULVERT
(Symbol)	CONCRETE PIPE
(Symbol)	CAST IRON PIPE
(Symbol)	STEEL PIPE
(Symbol)	WOOD PIPE
(Symbol)	CONCRETE ROAD CURB
(Symbol)	CAST IRON ROAD CURB
(Symbol)	STEEL ROAD CURB
(Symbol)	WOOD ROAD CURB
(Symbol)	CONCRETE SIDEWALK
(Symbol)	CAST IRON SIDEWALK
(Symbol)	STEEL SIDEWALK
(Symbol)	WOOD SIDEWALK
(Symbol)	CONCRETE DRIVEWAY
(Symbol)	CAST IRON DRIVEWAY
(Symbol)	STEEL DRIVEWAY
(Symbol)	WOOD DRIVEWAY
(Symbol)	CONCRETE FENCE
(Symbol)	CAST IRON FENCE
(Symbol)	STEEL FENCE
(Symbol)	WOOD FENCE
(Symbol)	CONCRETE UTILITY
(Symbol)	CAST IRON UTILITY
(Symbol)	STEEL UTILITY
(Symbol)	WOOD UTILITY



PROFILE
SCALE: VERTICAL 1"=2'

PLAN
SCALE: HORIZONTAL 1"=20'

NO.	DESCRIPTION	DATE
1	PREPARED FOR DESIGN	
2	DESIGN	
3	REVISIONS TO CONSTRUCTION DOCUMENTS	
4	CONSTRUCTION DOCUMENTS	
5	CONSTRUCTION DOCUMENTS	
6	CONSTRUCTION DOCUMENTS	
7	CONSTRUCTION DOCUMENTS	
8	CONSTRUCTION DOCUMENTS	
9	CONSTRUCTION DOCUMENTS	
10	CONSTRUCTION DOCUMENTS	

REVISIONS

REVISION 1
DATE: 11/14/11
BY: JAV
DESCRIPTION: BEAVER CREEK CONVEYANCE IMPROVEMENTS
LUMEN PUMPING STATION ADJUSTMENTS
SOUTH COAST, PENNSYLVANIA
PLAN / PROFILE 20
CER Engineering Services
10000 W. 10th Street, Suite 100, Golden, CO 80401
303.440.1000
www.cer-engineering.com



SCALE: 1"=20'