

GBBP BALTIMORE NEW DOLPHIN

4701 NEWGATE AVE., BALTIMORE, MD 21224

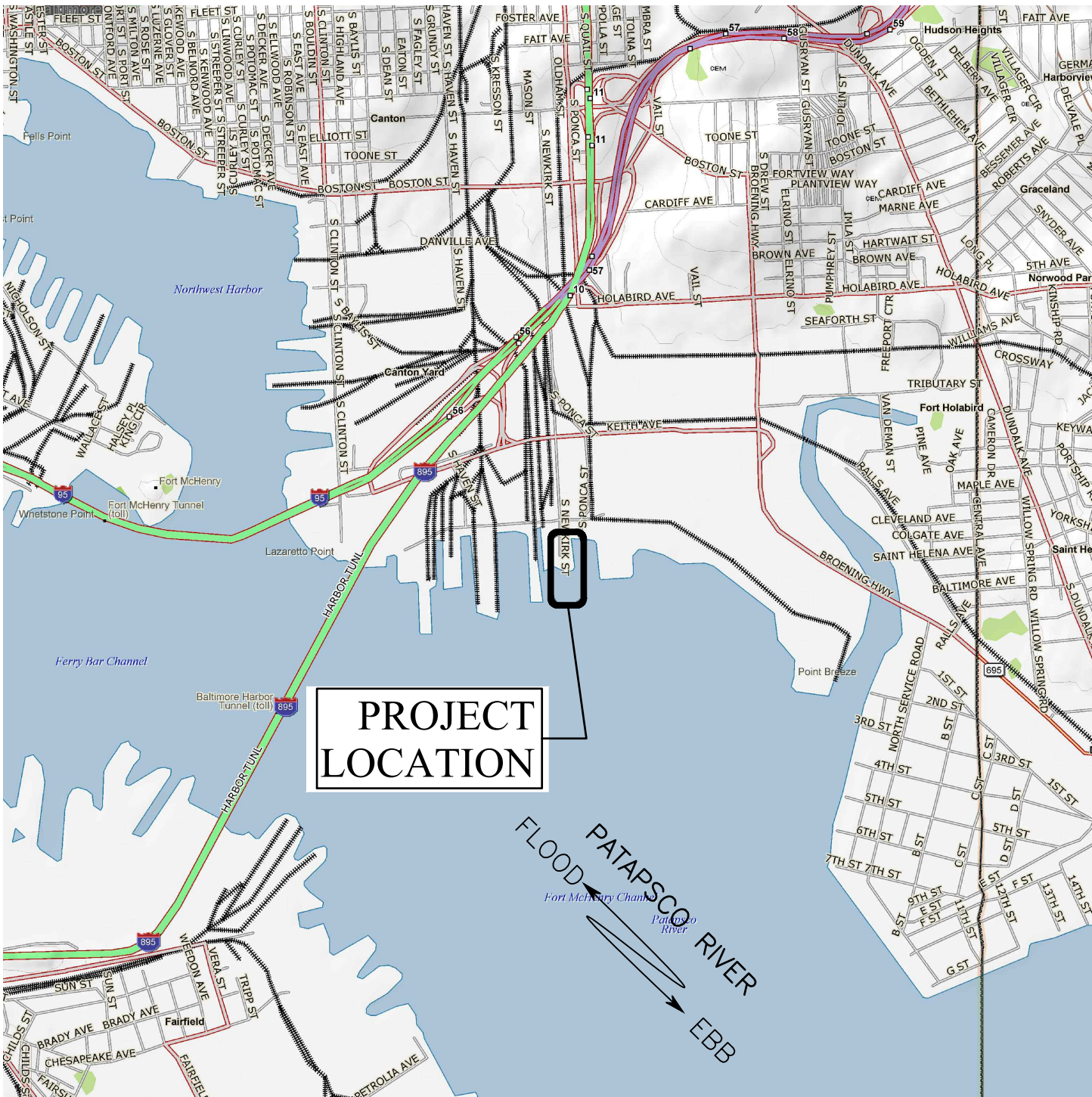
FOR

GOLD BOND BUILDING PRODUCTS, LLC

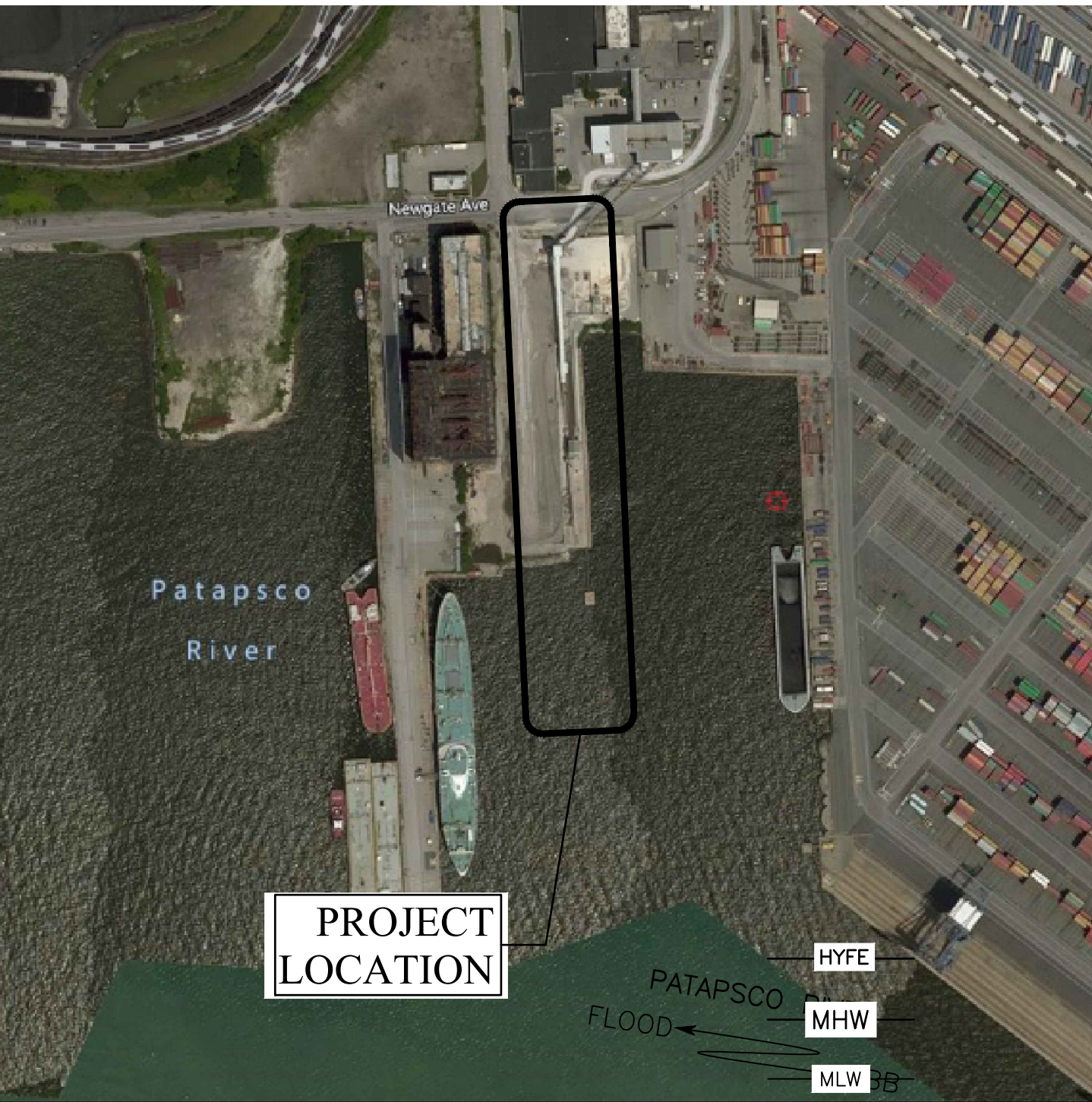
2301 S. NEWKIRK ST., BALTIMORE, MD 21224

NAB-2021-61797
Revised 2/2/22
Sheet 1 of 15

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VICINITY MAP



LOCATION MAP

URBAN PROJECT NO. 2020610213.000

ISSUED FOR
PERMIT REVISION
DATE: 02/02/22



SEAL

BY:

DATE:

REV	DATE	DESCRIPTION	BY	CHKD	APD
1	02/02/22	ISSUED FOR PERMIT REVISION	MKP	PB	MGW
0	06/16/21	ISSUED FOR PERMIT	MKP	PB	MGW

GOLD BOND BUILDING PRODUCTS, LLC		BALTIMORE, MD
GBBP BALTIMORE NEW DOLPHIN		COVER SHEET
SCALE: AS NOTED	DESIGN BY: MKP	
DATE: 02/02/22	APPROVED BY: MGW	
PROJECT NO: 2020610213.000	DRAWN BY: FVC	
SHEET NUMBER: CS-1	CHECKED BY: SMM	
SHT NO: SHT 1 OF TOT SH		
DRAWING FILE NO: CS-1 COV... .DWG		
REV NO: 1		

1. DESIGN AND CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE STATE AND LOCAL CODES AND STANDARDS, THE PROJECT SPECIFICATIONS, AND DRAWINGS. NOTES ON DRAWINGS SHALL TAKE PRECEDENCE OVER PROJECT SPECIFICATIONS. SPECIFIC NOTES ON DETAIL DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTE DRAWINGS. STATE AND LOCAL CODES SHALL TAKE PRECEDENCE OVER PROJECT SPECIFICATIONS AND NOTES ON DRAWINGS.
2. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS PRIOR TO MOBILIZING. ALL EXISTING & PROPOSED DIMENSIONS AND ELEVATIONS SHOWN ON THE DRAWINGS SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO ANY FABRICATION OR ERECTION. NOTIFY THE ENGINEER OF ANY DISCREPANCIES IMMEDIATELY.
3. THE CONTRACTOR SHALL REMOVE FROM THE SITE ALL WASTE MATERIAL, DEMOLISHED MATERIAL AND DEBRIS GENERATED DURING THE COURSE OF THE WORK, U.N.O. DISPOSAL OF ALL GENERATED WASTE MATERIAL, DEMOLISHED MATERIAL AND DEBRIS IS THE CONTRACTOR'S RESPONSIBILITY. DISPOSE OF ALL MATERIAL IN ACCORDANCE WITH APPLICABLE FEDERAL, STATE AND LOCAL RULES AND REGULATIONS.
4. THE CONTRACTOR SHALL PLACE SEDIMENT CONTROL DEVICES, BOOMS, TARPAULINS, FLOATS, STAGING, TURBIDITY BARRIERS, AND OTHER DEVICES AS NECESSARY TO PREVENT CONSTRUCTION MATERIALS FROM ENTERING THE WATER AND LEAVING THE IMMEDIATE VICINITY OF THE SITE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CLEANUP OF ALL MATERIALS.
5. THE CONTRACTOR SHALL ABIDE BY ALL APPLICABLE ENVIRONMENTAL PROTECTION STANDARDS, CODES, LAWS, REGULATIONS, AND PERMITS.
6. THE CONTRACTOR SHALL CONDUCT HIS OPERATIONS SO AS TO NOT INTERFERE WITH, OR BE DETRIMENTAL TO, VESSEL AND VEHICULAR TRAFFIC DURING THE COURSE OF THE WORK.
7. ALL STRUCTURES ARE DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER ERECTION IS FULLY COMPLETE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE ERECTION PROCEDURES AND SEQUENCE AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIE DOWNS THAT MAY BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER COMPLETION OF THE PROJECT.
8. THE CONTRACTOR SHALL FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

1. ALL CONCRETE WORK SHALL COMPLY WITH THE RECOMMENDATIONS OF ACI 301 AND ACI 318, UNLESS OTHERWISE SPECIFIED.
2. FOLLOW THE REQUIREMENTS OF ACI 306 WHEN POURING CONCRETE IN COLD WEATHER.
3. CAST-IN-PLACE CONCRETE STRENGTH SHALL BE A MINIMUM OF 4500 PSI AT 28 DAYS, U.N.O.
4. NONMETALLIC NON-SHRINK GROUT STRENGTH SHALL BE A MINIMUM OF 5000 PSI AT 28 DAYS, U.N.O.
5. THE CONTRACTOR SHALL MAINTAIN PROPER CURING PROCEDURES FOR A MINIMUM OF 7 DAYS.
6. ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING STEEL SHALL CONFORM TO THE ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, ACI 315.
7. REINFORCING STEEL FOR CAST-IN-PLACE CONCRETE SHALL CONFORM TO ASTM A 615, GR 60, EXCEPT REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A 706.
8. MINIMUM CONCRETE COVER FOR ALL REINFORCING SHALL BE 2 INCHES, U.N.O.
9. ALL CONSTRUCTION JOINTS SHALL BE KEYED, U.N.O. JOINTS SHALL BE PROVIDED AS NECESSARY AT LOCATIONS, AND USING METHODS, APPROVED BY THE ENGINEER.
10. ALL REINFORCING BAR SPLICES SHALL BE TENSION LAP SPLICES, IN ACCORDANCE WITH ACI 318, U.N.O.
11. CHAMFER ALL EXTERNAL EXPOSED CORNERS OF CONCRETE WITH A 1 INCH, 45-DEGREE CHAMFER, U.N.O.
12. WELDING OF REINFORCING STEEL SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.4.

1. ALL STEEL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL. DESIGN, FABRICATION, AND ERECTION ARE TO BE GOVERNED BY THE LATEST REVISIONS OF:
 - A. AISC STEEL CONSTRUCTION MANUAL
 - B. AISC CODE OF STANDARD PRACTICE
 - C. AWS D1.1 STRUCTURAL WELDING CODE
 - D. RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS
2. STRUCTURAL AND MISCELLANEOUS STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING, U.N.O.:

WIDE FLANGE SHAPES:	ASTM A 992, GRADE 50
HP PILES:	ASTM A 572, GRADE 50
PIPE PILES:	ASTM A 252, GRADE 3, $F_y = 50$

KSI

PIPE:	ASTM A 53, GRADE B, TYPE S
HOLLOW STRUCTURAL SECTIONS (HSS):	ASTM A 500
OTHER SHAPES, BARS AND PLATES:	ASTM A 36, $F_y = 36$ KSI
HEADED BOLTS:	ASTM A 325
ANCHOR BOLTS:	ASTM A 449
STEEL SHEET PILE (SSP):	ASTM A 572, GRADE 50
NUTS:	ASTM A 563
WASHERS:	ASTM F 436
3. IF OVERSIZED HOLES ARE USED, WASHERS MUST BE EXTRA THICK, AND MAY REQUIRE WELDING.
4. ALL MISCELLANEOUS METALS INCLUDING BOLTS, WASHERS, NUTS, ANGLES, INSERTS, PLATES, AND OTHER FABRICATIONS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM A 123 AND/OR ASTM A 153, AS APPLICABLE AFTER FABRICATION, UNLESS OTHERWISE NOTED OR SPECIFIED. STAINLESS STEEL ITEMS SHALL NOT BE GALVANIZED.
5. FIELD TREAT DAMAGED GALVANIZED FINISH WITH TWO COATS OF HIGH ZINC DUST OXIDE PAINT, COLD GALVANIZING COMPOUNDS OR APPROVED EQUAL. IN ADDITION, ALL EXPOSED THREADED SURFACES SHALL BE CLEANED AND PAINTED WITH TWO COATS OF HIGH ZINC DUST OXIDE PAINT AFTER INSTALLATION OF THE NUT(S).
6. THE TYPE, SIZE, SPACING, AND ALIGNMENT OF CONNECTING HARDWARE SHOWN ARE CRITICAL AND MUST BE MAINTAINED.
7. ALL WELDING SHALL CONFORM TO THE LATEST EDITION OF AWS D1.1, U.N.O. ALL WELDS SHALL BE MADE WITH E70XX ELECTRODES, UNLESS HIGHER STRENGTH ELECTRODES ARE REQUIRED BY AWS D1.1. GENERALLY, ELECTRODES SHALL BE NEW OR BAKED AT THE START OF EACH SHIFT. FOLLOW THE ATMOSPHERIC EXPOSURE LIMITS AND BAKING REQUIREMENTS OF CLAUSE 5 OF AWS D1.1.
8. WELDING PERSONNEL AND PROCEDURES ARE TO BE QUALIFIED PER AWS D1.1, U.N.O.
9. MINIMUM SIZE OF FILLET WELDS SHALL BE $\frac{5}{16}$ " U.N.O.
10. WHEN THE 'ALL-AROUND' WELDING SYMBOL IS USED AT AN END CONNECTION OF A ROLLED SHAPE, THE CONTRACTOR SHALL TERMINATE THE WELD AT EACH OF THE FLANGE EDGES, AND ONE WELD THICKNESS AWAY FROM EACH INTERSECTION OF THE WEB AND FLANGE, U.N.O.
11. DOWELS, BOLTS, REINFORCEMENT OR OTHER STEEL TO BE EMBEDDED IN CONCRETE SHALL BE INSTALLED USING HILTI HIT-HY 200 EPOXY, U.N.O.

1. LAND SURVEY INFORMATION SHOWN ON THESE PLAN IS BASED ON A FIELD SURVEY, WHICH WAS CONDUCTED BY NAVARRO & WRIGHT CONSULTING ENGINEERS. DURING THE MONTH OF OCTOBER, 2020.
2. DATUM:
HORIZONTAL = MARYLAND STATE PLANE COORDINATE SYSTEM, SOUTH ZONE, NAD83
VERTICAL = MLW (SURVEY WAS CONDUCTED IN NAVD88 VERTICAL DATUM AND CONVERTED TO MLW DATUM IN ACCORDANCE WITH ELEVATION LEGEND).
3. HYDROGRAPHIC SURVEY INFORMATION SHOWN ON THESE PLANS IS BASED ON A HYDROGRAPHIC SURVEY, WHICH WAS CONDUCTED BY GAHAGAN & BRYANT ASSOCIATES, INC ON NOVEMBER 18, 2020.

ASHTO	=	AMERICAN ASSOCIATION OF STATE HIGHWAY	LT	=	LONG TON
		AND TRANSPORTATION OFFICIALS	m	=	METERS
ACI	=	AMERICAN CONCRETE INSTITUTE	mm	=	MILLIMETERS
AISC	=	AMERICAN INSTITUTE OF	MAX	=	MAXIMUM
		STEEL CONSTRUCTION	MEA	=	MEASURED
ALT	=	ALTERNATE	MHHW	=	MEAN HIGHER HIGH WATER
ANSI	=	AMERICAN NATIONAL	MHW	=	MEAN HIGH WATER
		STANDARDS INSTITUTE	MIN	=	MINIMUM
APPROX	=	APPROXIMATE	MISC	=	MISCELLANEOUS
ARCH.	=	ARCHITECTURAL	MLLW	=	MEAN LOWER LOW WATER
ASTM	=	AMERICAN SOCIETY FOR	MLW	=	MEAN LOW WATER
		TESTING AND MATERIALS	MPa	=	MEGAPASCALS
AVG	=	AVERAGE	MPH	=	MILES PER HOUR
AWS	=	AMERICAN WELDING SOCIETY	MSL	=	MEAN SEA LEVEL
B.O.P.	=	BOTTOM OF PIPE	N	=	NEWTON OR NORTH
B/B	=	BACK TO BACK	NAVD-88	=	NORTH AMERICAN VERTICAL
C/C	=	CENTER TO CENTER			DATUM OF 1988
CF	=	CUBIC FEET	NGVD-29	=	NATIONAL GEODETIC VERTICAL
C.I.P.	=	CAST IN PLACE			DATUM OF 1929
CJ	=	CONSTRUCTION JOINT	NTP	=	NOTICE TO PROCEED
CLR	=	CLEAR	NIC	=	NOT IN CONTRACT
CO	=	CLEAN OUT	NS	=	NEAR SIDE
CONC	=	CONCRETE	NTS	=	NOT TO SCALE
CONST	=	CONSTRUCTION	NO.	=	NUMBER
CONT	=	CONTINUOUS	O.C.	=	ON CENTER
CTE	=	COAL TAR EPOXY	O/S	=	OUTSHORE
CY	=	CUBIC YARD	OSHA	=	OCCUPATIONAL SAFETY AND
D	=	DEPTH			HEALTH ADMINISTRATION
D/R	=	DOWNRIVER	PSI	=	POUNDS PER SQUARE INCH
DEG	=	DEGREES	PVC	=	POLYVINYL CHLORIDE
DET'L'S	=	DETAILS	R/F	=	REINFORCING
DIA	=	DIAMETER	R.C.P.	=	REINFORCED CONCRETE PIPE
DIST	=	DISTANCE	REQ'D	=	REQUIRED
DWG	=	DRAWING	S	=	SOUTH
DWT	=	DEAD WEIGHT TONNAGE	SCH	=	SCHEDULE
E	=	EAST	SECT	=	SECTION
EA	=	EACH	SHT	=	SHEET
EF	=	EACH FACE	SIM	=	SIMILAR
EHW	=	EXTREME HIGH WATER	SPEC	=	SPECIFICATION
EJ	=	EXPANSION JOINT	SQ	=	SQUARE
EL/ELEV.	=	ELEVATION	SS	=	STAINLESS STEEL
ELEC	=	ELECTRICAL	SSP	=	STEEL SHEET PILE
EW	=	EACH WAY	STL	=	STEEL
EXIST.	=	EXISTING	STRUC.	=	STRUCTURE
EXT	=	EXTERIOR	SUPP	=	SUPPORT
EXPN	=	EXPANSION	SY	=	SQUARE YARD
FDN.	=	FOUNDATION	T, TONNE	=	METRIC TON
FPS	=	FEET PER SECOND	TON	=	SHORT TON
FRP	=	FIBERGLASS REINFORCED PLASTIC	T.B.D	=	TO BE DETERMINED
FS	=	FAR SIDE	TEMP	=	TEMPORARY
FT	=	FEET	THK	=	THICKNESS
FY	=	YIELD STRESS	THRU	=	THROUGH
GALV	=	GALVANIZED	T.O.	=	TOP OF
GR	=	GRADE	T.O.C	=	TOP OF CONCRETE
GRING	=	GRATING	T.O.S	=	TOP OF STEEL
GUSS	=	GUSSET	TYP	=	TYPICAL
H	=	HEIGHT	U.N.O.	=	UNLESS NOTED OTHERWISE
HAT	=	HIGHEST ASTRONOMICAL TIDE	UHMWPE	=	ULTRA HIGH MOLECULAR WEIGHT
HTL	=	HIGH TIDE LINE			POLYETHYLENE
HVY	=	HEAVY	U/R	=	UP RIVER
I	=	I BEAM	USACE	=	UNITED STATES ARMY CORPS
I/S	=	INSHORE			OF ENGINEERS
INV	=	INVERT	W/	=	WITH
JT	=	JOINT	W	=	WIDTH, WIDE FLANGE BEAM OR WEST
kg	=	KILOGRAMS	W.P./WP	=	WORK POINT
KIP	=	KILOPOUND	W.T.	=	WALL THICKNESS
kN	=	KILONEWTONS	WT	=	WEIGHT
L	=	ANGLE	℄	=	CENTER LINE
LAT	=	LOWEST ASTRONOMICAL TIDE OR	#	=	NUMBER
		LATITUDE	%	=	PERCENT
LB	=	POUND	℄	=	PLATE
LF	=	LINEAR FEET	&	=	AND
LG	=	LONG	@	=	AT
LLV	=	LONG LEG VERTICAL	x	=	BY
LOA	=	LENGTH OVERALL	ø	=	DIAMETER
LONG.	=	LONGITUDE			

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T:\PROJECTS\ON-GOING\2020 PROJECTS\2020\610213.000 - NGC BALTIMORE NEW DOLPHINWORK\DRAWINGS\WORKING DRAWINGS\CURRENT\GN-1 GENERAL NOTES.DWG

DESIGN VESSELS

1. LARGEST DESIGN VESSEL CHARACTERISTICS:
- MAX. DWT

MAX. DISPLACEMENT

MAX. DESIGN DISPLACEMENT

LOA = 750 FT

BEAM = 106 FT

MAX. DESIGN DRAFT (PARTIAL)

MIN. BALLASTED DESIGN DRAFT
- =

=

=

=

=
- 71,405 TONNES

87,500 TONNES

62,000 TONNES

33 FT

22 FT

DESIGN LOADS

1. LIVE LOADS:
- WALKWAY AND DOLPHIN LIVE LOAD

=

100 PSF
2. WIND:
- MAXIMUM OPERATING WIND VELOCITY

STORM WIND VELOCITY (PERMANENT STRUCTURE ONLY)

RISK CATEGORY

EXPOSURE CATEGORY

BASE WIND VELOCITY
- =

=

=
- 50 KNOTS (58 MPH)

II

C

115 MPH
3. CURRENT:
- MAX CURRENT VELOCITY

=

2.5 KNOTS
4. SEISMIC:
- RISK CATEGORY

SITE CLASS

S_s

S_i
- =

=

=

=
- II

D

0.129g

0.052g

FENDERS

1. FENDERS SHALL BE MARITIME INTERNATIONAL 1600 CELL FENDER, GRADE 2, OR APPROVED EQUAL. FENDERS SHALL BE EQUIPPED WITH UHWM-FACED PANELS. THE CONTRACTOR SHALL SUBMIT PROPOSED FENDER SPECIFICATIONS FOR APPROVAL PRIOR TO PROCUREMENT.
2. FENDERS SHALL HAVE THE FOLLOWING RATED PROPERTIES, U.N.O.:
- MINIMUM ENERGY

MAXIMUM REACTION

DEFLECTION
- =

=

=
- 755 K-FT

400 KIP

52.5%
3. BERTHING PARAMETERS:
- MAXIMUM BERTHING VELOCITY

ABNORMAL IMPACT FACTOR

MAXIMUM BERTHING ANGLE

SERVICE TEMPATURE RANGE
- =

=

=

=
- 0.50 FT./SEC.

1.50

3°

30-100 °F
4. ALL CHAINS, ANCHORAGES, SHACKLES, PANELS AND HARDWARE SHALL BE DESIGNED AND SUPPLIED BY THE FENDER MANUFACTURER. FENDER PANELS SHALL BE THE APPROXIMATE SIZES SHOWN AND SHALL LIMIT HULL PRESSURES TO 20 PSI. THE CONTRACTOR SHALL SUBMIT PROPOSED SHOW DRAWINGS FOR THE ABOVE FOR REVIEW PRIOR TO PROCUREMENT.

PILE TESTING & DRIVE CRITERIA

1. STANDARD PILES SHALL BE DRIVEN TO ACHIEVE THE FOLLOWING ALLOWABLE CAPACITIES:
- 1.1. 18"ø×⁵/₈" PLUMB PILES SHALL BE DRIVEN TO ACHIEVE A 65 TON ALLOWABLE CAPACITY IN COMPRESSION, AND A 38 TON ALLOWABLE CAPACITY IN UPLIFT.
- 1.2. 18"ø×⁵/₈" BATTER PILES SHALL BE DRIVEN TO ACHIEVE A 120 TON ALLOWABLE BEARING CAPACITY IN COMPRESSION, AND A 5 TON ALLOWABLE CAPACITY IN UPLIFT
- 1.3. THE MINIMUM FACTORS OF SAFETY ON THESE ALLOWABLE CAPACITIES SHALL BE 2.25 ON COMPRESSION, AND 3.0 ON UPLIFT.
2. THE CONTRACTOR SHALL PERFORM AND SUBMIT THE RESULTS OF DRIVEABILITY ANALYSES BASED ON BORING B-1 OF THE GEOTECH REPORT TO DETERMINE THE SPECIFICATIONS OF THE HAMMER TO BE USED FOR DRIVING STANDARD PILES TO THE REQUIRED ALLOWABLE CAPACITIES. THIS HAMMER WILL BE REVIEWED BY THE ENGINEER PRIOR TO USE.
3. THE CONTRACTOR SHALL PERFORM AND SUBMIT THE RESULTS OF DYNAMIC TESTING AND CAPWAP ANALYSES OF THE TEST PILES INDICATED ON THE DWGS. THE DYNAMIC TESTING SHALL INCLUDE PDA MONITORING DURING INITIAL DRIVE, AND A RESTRIKE TO BE PERFORMED AFTER A MINIMUM OF 48 HOURS.
- 3.1. TEST PILES SHALL BE DRIVEN AND TESTED FOR THEIR FULL LENGTHS.
- 3.2. THIS TESTING SHALL INFORM THE DEVELOPMENT OF DRIVE CRITERIA TO ACHIEVE THE SPECIFIED ALLOWABLE PILE CAPACITIES IN COMPRESSION AND UPLIFT. THE ENGINEER WILL ISSUE DRIVE CRITERIA BASED ON THE RESULTS FROM THE TEST PILES, WHICH SHALL GOVERN THE DRIVING OF PRODUCTION PILES.
- 3.3. PRODUCTION PILES OF EACH TYPE SHALL NOT BE DRIVEN UNTIL THAT TYPE HAS BEEN TESTED AND THE ENGINEER HAS ISSUED DRIVE CRITERIA FOR THAT TYPE.
4. SEE THE TECHNICAL SPECIFICATIONS FOR FURTHER DETAILS ON PILE TESTING REQUIREMENTS.

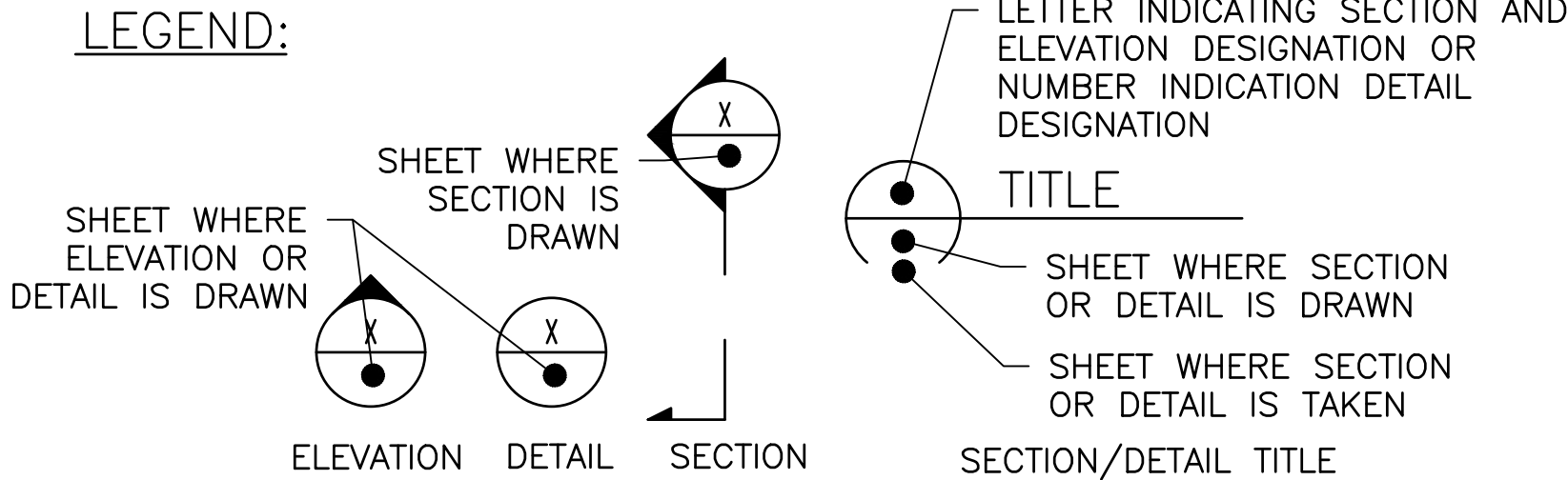
BOLLARDS

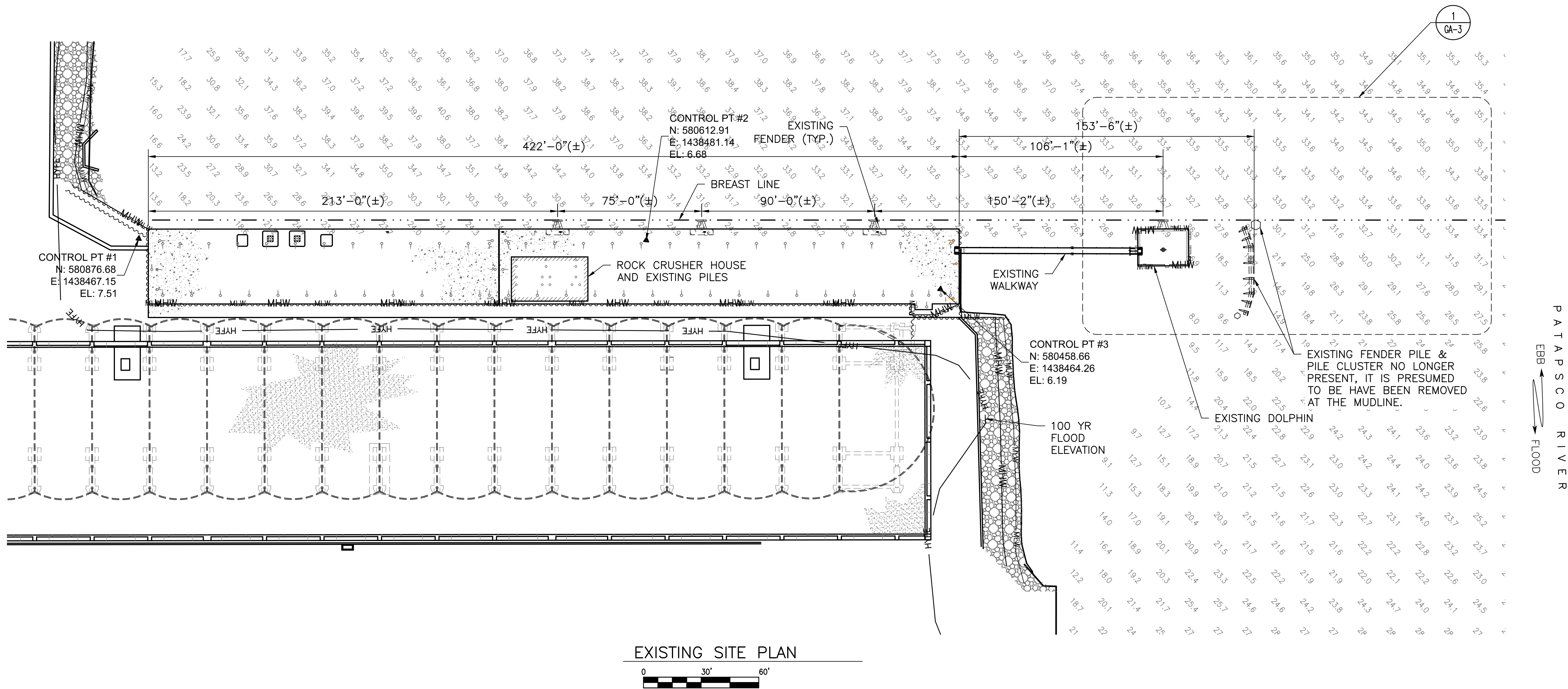
1. BOLLARDS SHALL BE 100 TON CAPACITY, SINGLE BITT STYLE BOLLARDS.
2. ANCHOR BOLTS SHALL BE DESIGNED AND SUPPLIED BY THE MANUFACTURER.
3. SUBMIT PROPOSED BOLLARD AND ANCHOR BOLT SHOP DRAWINGS FOR REVIEW PRIOR TO PROCUREMENT, AND INSTALL BOLLARDS USING THE MANUFACTURER’S RECOMMENDATIONS, AS APPROVED BY THE ENGINEER.

FOUNDATION

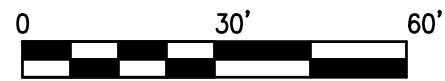
1. THE CONTRACTOR SHALL BECOME FAMILIAR WITH THE AVAILABLE SOIL BORING INFORMATION. IF AVAILABLE, THE GEOTECHNICAL INVESTIGATION REPORT FOR THIS PROJECT CAN BE OBTAINED FROM THE OWNER.
2. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY UNUSUAL SOIL CONDITIONS IMMEDIATELY.
3. THE CONTRACTOR SHALL TAKE CARE WHEN USING HEAVY EQUIPMENT IN THE VICINITY OF EXCAVATED SLOPES. NO WORK SHALL BE PERFORMED IN THE VICINITY OF SLOPES THAT ARE SATURATED FROM HEAVY RAINS, OR THROUGH WHICH GROUNDWATER IS ACTIVELY DRAINING.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, INSTALLATION AND FINAL CLEARANCE OF ANY REQUIRED NEEDLING, UNDERPINNING, SHORING, FALSEWORK, FORMWORK, TEMPORARY SUPPORT, AND TEMPORARY BRACING OF EXISTING STRUCTURES.
5. SPUDDING, PRE-AUGERING, AND/OR JETTING OF PILES IS NOT PERMITTED, U.N.O.
6. TIMBER PILES SHALL CONFORM TO ASTM D 25. TIMBER PILES SHALL BE UNTREATED SOUTHERN YELLOW PINE, U.N.O.
7. DO NOT DRIVE, VIBRATE, OR EXTRACT PILES WITHIN 50 FT. OF RECENTLY POURED CONCRETE UNTIL THE CONCRETE HAS BEEN PROPERLY CURED FOR A MINIMUM OF 7 DAYS.
8. EXISTING AND NEW STRUCTURES SHALL NOT BE USED AS FALSEWORK FOR PILE DRIVING, U.N.O.
9. STEEL PIPE PILES SHALL BE WELDABLE IN ACCORDANCE WITH AWS D1.1. SPIRAL WELD PIPE PILES SHALL ONLY BE USED WITH THE WRITTEN APPROVAL OF THE ENGINEER.
10. PILES SHALL NOT BE PULLED, JACKED, OR OTHERWISE MANIPULATED INTO POSITION. THE CONTRACTOR SHALL ENSURE THAT ALL PILES ARE INSTALLED IN THE LOCATIONS INDICATED ON THE DRAWINGS. THE CONTRACTOR MUST COORDINATE WITH THE ENGINEER AND ALLOW THE ENGINEER TO MEASURE THE PILE POSITION BEFORE WELDING OR CONNECTING DRIVEN PILES, AT THE ENGINEER’S DISCRETION. THE FINAL HORIZONTAL TOLERANCE ALLOWED AT THE TOPS OF THE PILE IS 1” MAXIMUM. IF A PILE IS OUTSIDE OF THIS TOLERANCE AFTER DRIVING AND FALSEWORK REMOVAL, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SUBMIT A MODIFIED CONNECTION DETAIL TO THE ENGINEER FOR APPROVAL.

REBAR TABLE				
BAR SIZE	MIN. LAP SPLICE		MIN. DEVELOPMENT LENGTH	
	TOP BAR (INCHES)	OTHER BAR (INCHES)	TOP BAR (INCHES)	OTHER BAR (INCHES)
#4	19	16	15	12
#5	24	19	19	14
#6	29	22	22	17
#7	42	32	32	25
#8	48	37	37	28
#9	60	46	46	35
#10	74	57	57	44
#11	88	68	68	52





EXISTING SITE PLAN



NAB-2021-61797
Revised 2/2/22
Sheet 4 of 15

ISSUED FOR
PERMIT REVISION
DATE: 02/02/22

LEGEND:

- HYPE — 100 YEAR FLOOD ELEVATION
- MHW — MEAN HIGH WATER LINE
- MLW — MEAN LOW WATER LINE

NOTES:

- FOR GENERAL NOTES, SEE GN-1 & GN-2.
- SOME OF THE CONVEYOR STRUCTURES ON THE MAIN PIER ARE NOT SHOWN FOR CLARITY.

Urban Engineers, Inc.
3500 West Street
Pawcatuck, CT 06259
6.21.22.360

SEAL

BY: _____

DATE: _____

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1	02/02/22	ISSUED FOR PERMIT REVISION	MP	PB	MP
0	06/16/21	ISSUED FOR PERMIT	MP	PB	MP

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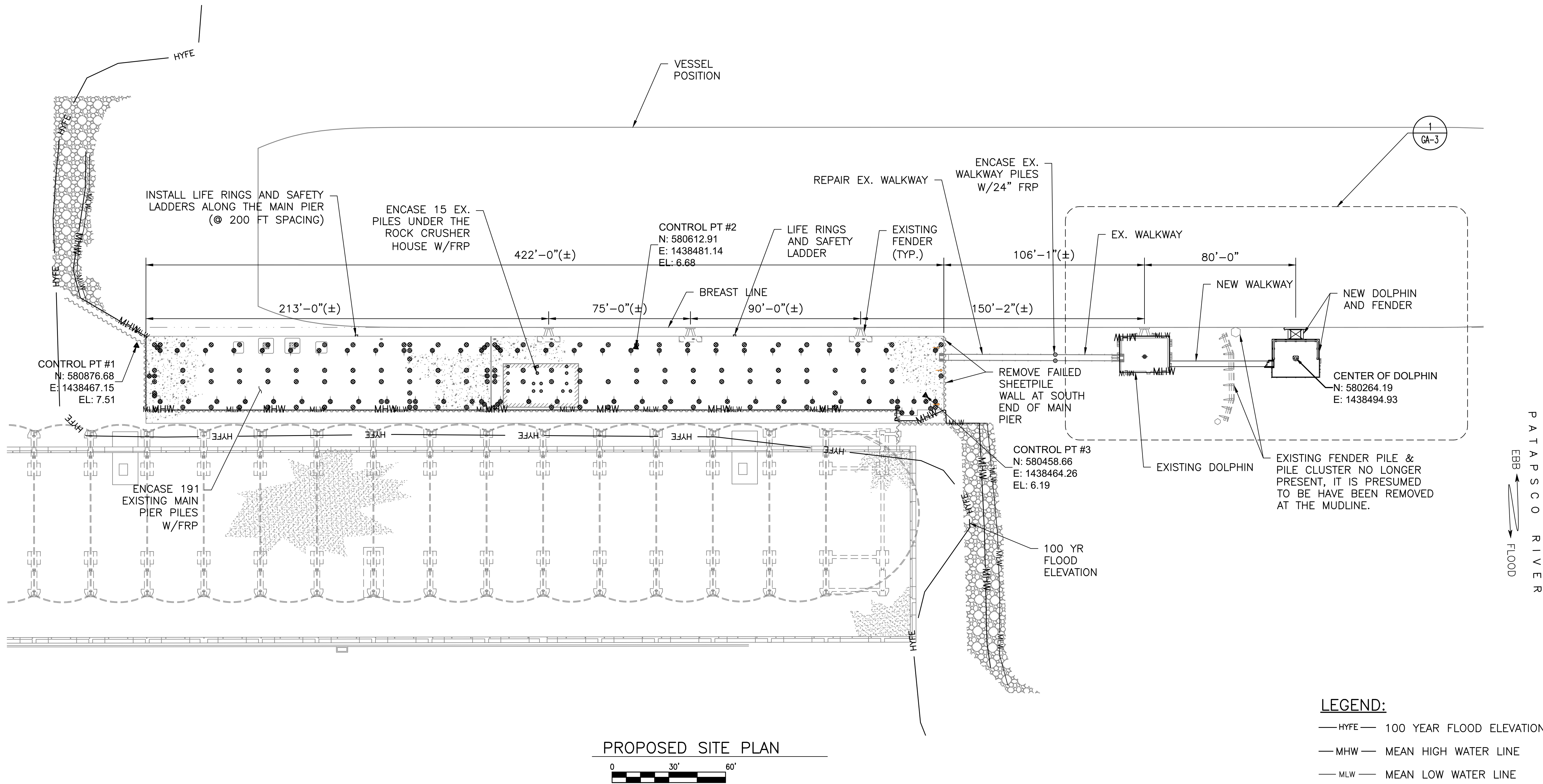
BALTIMORE, MD

GBBP BALTIMORE NEW DOLPHIN

EXISTING SITE PLAN

SCALE:	AS NOTED	DESIGN BY:	MP
DATE:	02/02/22	APPROVED BY:	MP
PROJECT NO.:	2020610213.000	DRAWN BY:	FIC
SHEET NUMBER:	GA-1	CHECKED BY:	SM

SHT NO.:	SHT 1	OF TOT SH
DRAWING FILE NO.:	GA-1 EXI... .DWG	REV NO.:
		1



LEGEND	DESCRIPTIONS	OVERALL SIZE/MAX. LxW	CROSS SECTION AREA, SF	OVERALL QUANTITIES	PERMANENT IMPACT	TEMPORARY IMPACT
					WATERCOURSE (MAX. SIZE, AREA (SF))	WATERCOURSE (SIZE, AREA (SF))
	PIPE PILE W/ENCASEMENT	24" DIA.	3.14	10	31.4	
	BATTER PILE W/ENCASEMENT	24" DIA.	3.14	8	25.1	
	GANGWAY	54.5'x3.75'	204.37	1	204.4	
	TURBIDITY CURTAIN					196'x141', 24024
	BREASTING DOLPHIN	20'x25'	500	1	500	
	REPAIR JACKETED PILE	30"ø	4.90	191	936	
	REPAIR JACKETED PILE	18"ø	1.77	15	16.8	
	REPAIR JACKETED FOR HP PILE	24"ø	3.14	2	6.3	
	TOTAL (SF)				1720	24024

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- LEGEND:**
- HYFE — 100 YEAR FLOOD ELEVATION
 - MHW — MEAN HIGH WATER LINE
 - MLW — MEAN LOW WATER LINE
- NOTES:**
- FOR GENERAL NOTES, SEE GN-1 & GN-2.
 - NOTE THAT SOME CONVEYOR STRUCTURES ON THE MAIN PIER ARE NOT SHOWN FOR CLARITY.
 - THE CONTRACTOR SHALL SURVEY EXISTING AND PROPOSED STRUCTURES TO ESTABLISH THE NEW FENDER LINE, AND SUBMIT THE RESULTS TO THE ENGINEER. THE NEW FENDER LINE MAYBE ADJUSTED SLIGHTLY, BASED ON THE EXACT ENDER AND PANEL DIMENSIONS, AND FIELD OBSTRUCTIONS, BUT MUST BOTH MAINTAIN ADEQUATE CLEARANCE FROM THE EXISTING FENDER LINE, AND KEEP HARD POINTS OF THE NEW DOLPHINS FROM CAUSING UNSAFE CONDITIONS DURING CONSTRUCTION.
 - THE CONTRACTOR'S FALSEWORK SHALL BE FLEXIBLE ENOUGH TO ACCOMMODATE SHIFTS OF UP TO 5 FT IN ORDER TO ADDRESS OBSTRUCTIONS OR FENDER DIMENSION, AS DISCUSSED ABOVE.

Urban Engineers, Inc.
350 Walnut Street
Philadelphia, PA 19106
215.722.3600

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DATE: _____

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0	06/16/21	ISSUED FOR PERMIT

GOLD BOND BUILDING PRODUCTS, LLC

GBBP BALTIMORE NEW DOLPHIN

PROPOSED SITE PLAN

SCALE: AS NOTED

DATE: 02/02/22

PROJECT NO: 2020610213.000

SHEET NUMBER: GA-2

SHT NO: SHT 1 OF TOT SH

DRAWING FILE NO: GA-2 PRO... .DWG

DESIGN BY: MGP

APPROVED BY: MGP

DRAWN BY: FIC

CHECKED BY: SMM

REV NO: 1

1

GA-3

EBB

FLOOD

PATAPSCO RIVER

100 YR FLOOD ELEVATION

CONTROL PT #1
N: 580876.68
E: 1438467.15
EL: 7.51

CONTROL PT #2
N: 580612.91
E: 1438481.14
EL: 6.68

CONTROL PT #3
N: 580458.66
E: 1438464.26
EL: 6.19

CENTER OF DOLPHIN
N: 580264.19
E: 1438494.93

EXISTING FENDER PILE & PILE CLUSTER NO LONGER PRESENT, IT IS PRESUMED TO BE HAD BEEN REMOVED AT THE MUDLINE.

REMOVE FAILED SHEETPILE WALL AT SOUTH END OF MAIN PIER

EXISTING DOLPHIN

NEW DOLPHIN AND FENDER

NEW WALKWAY

EX. WALKWAY

106'-1" (±)

80'-0"

150'-2" (±)

90'-0" (±)

75'-0" (±)

213'-0" (±)

422'-0" (±)

ENCASE 191 EXISTING MAIN PIER PILES W/FRP

ENCASE 15 EX. PILES UNDER THE ROCK CRUSHER HOUSE W/FRP

INSTALL LIFE RINGS AND SAFETY LADDERS ALONG THE MAIN PIER (@ 200 FT SPACING)

REPAIR EX. WALKWAY

ENCASE EX. WALKWAY PILES W/24" FRP

EXISTING FENDER (TYP.)

LIFE RINGS AND SAFETY LADDER

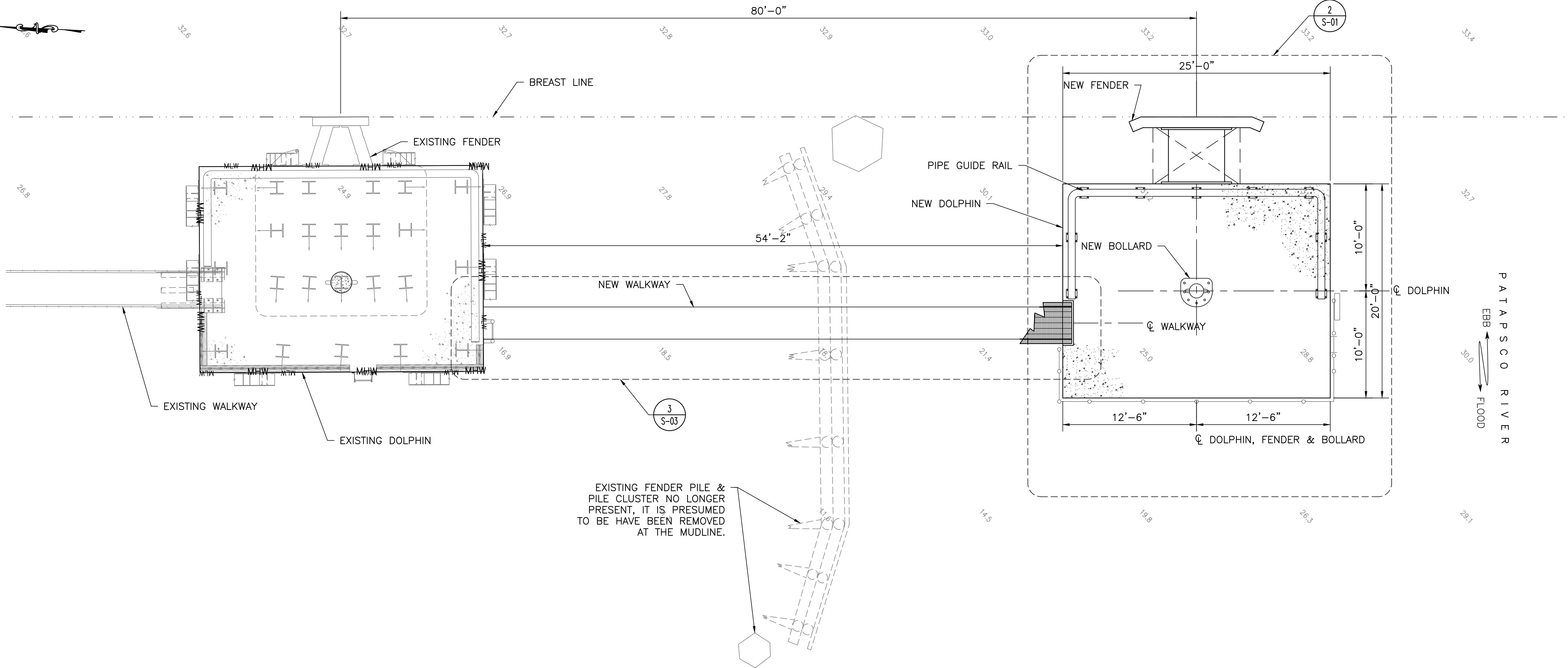
BREAST LINE

HYFE

MHW

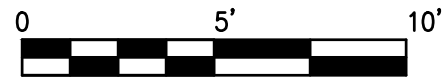
MLW

100 YR FLOOD ELEVATION



EXISTING FENDER PILE & PILE CLUSTER NO LONGER PRESENT, IT IS PRESUMED TO BE HAVE BEEN REMOVED AT THE MUDLINE.

1
GA-3
GA-1
PROPOSED DOLPHIN AND WALKWAY



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- LEGEND:**
- 100 YEAR FLOOD ELEVATION
 - MHW — MEAN HIGH WATER LINE
 - MLW — MEAN LOW WATER LINE

- NOTES:**
- FOR GENERAL NOTES, SEE GN-1 & GN-2.



Urban Engineers, Inc.
350 West 3rd Street
Baltimore, MD 21201
410.572.2600

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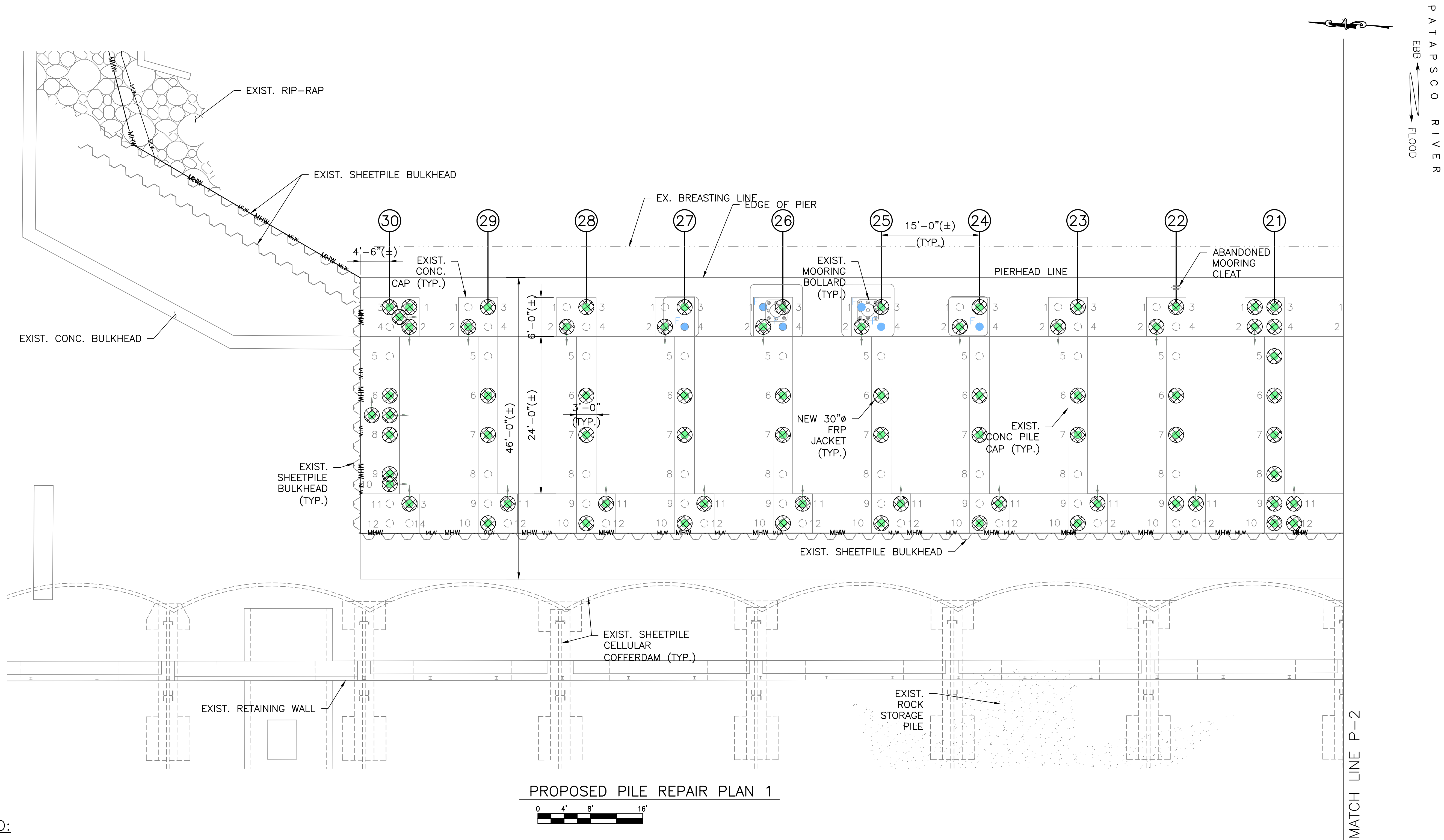
DATE:

REV	DATE	DESCRIPTION	BY	CHKD	APD
1	02/02/22	ISSUED FOR PERMIT REVISION	MKP	PB	MCW
0	06/16/21	ISSUED FOR PERMIT	MKP	PB	MCW

GOLD BOND BUILDING PRODUCTS, LLC	BALTIMORE, MD
GBBP BALTIMORE NEW DOLPHIN	
PROPOSED DOLPHIN AND WALKWAY	
SCALE: AS NOTED	DESIGN BY: MKP
DATE: 02/02/22	APPROVED BY: MCW
	DRAWN BY: FIC
	CHECKED BY: SMW
PROJECT NO: 2020610213.000	
SHEET NUMBER: GA-3	
SHT NO: SHT 1 OF TOT SH	
DRAWING FILE NO: GA-3 PRO... .DWG	REV NO: 1

LEGEND:

- (X) BENT NUMBER
- ABANDONED PLUMB PILE
- ABANDONED BATTER PILE
- JACKETED PILE
- F FIBERGLASS JACKETED PILE
- ▽ SEVERED PILE
- 36"Ø CONC. FENDER PILE
- ⊗ NEW 30"Ø FIBERGLASS REINFORCED PLASTIC PIPE (FRP.) JACKET



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1	02/02/22	ISSUED FOR PERMIT REVISION	MP	PB	MGW
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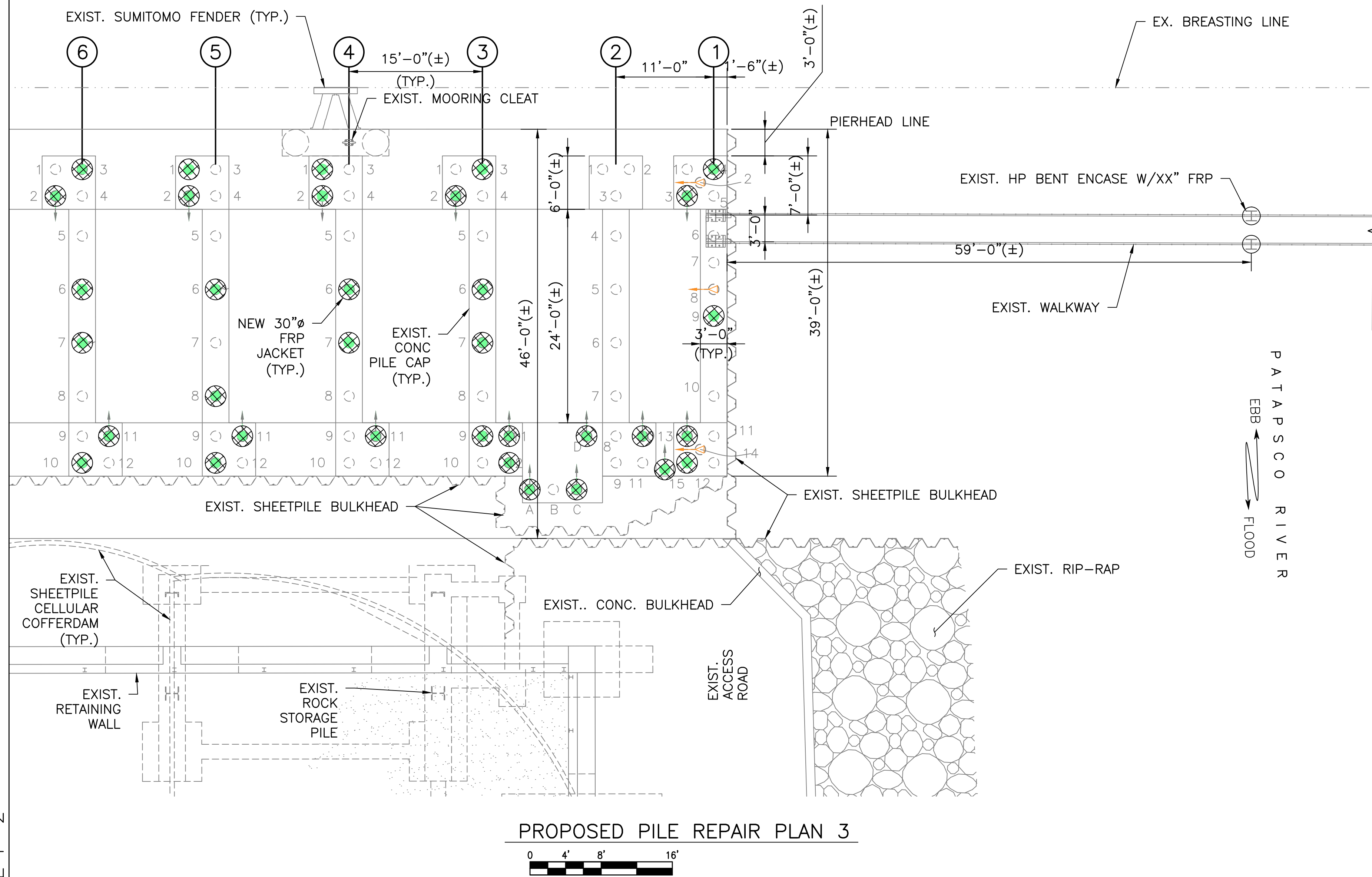
GOLD BOND BUILDING PRODUCTS, LLC

GBBP BALTIMORE NEW DOLPHIN

PROPOSED PILE REPAIR PLAN 1

SCALE:	AS NOTED	DESIGN BY:	MP
DATE:	02/02/22	APPROVED BY:	MGW
PROJECT NO.:	2020610213.000	DRAWN BY:	FJC
SHEET NUMBER:	P-1	CHECKED BY:	SNM
SHT NO.:	SHT 1	OF TOT SH	
DRAWING FILE NO.:	P-1 EXIS... .DWG	REV NO.:	1

MATCH LINE P-2



LEGEND:

- | | | | |
|---|--|---|-----------------------|
|  | BENT NUMBER |  | ABANDONED/BROKEN PILE |
|  | ABANDONED PLUMB PILE | | |
|  | ABANDONED BATTER PILE | | |
|  | JACKETED PILE | | |
|  | FIBERGLASS JACKETED PILE | | |
|  | SEVERED PILE | | |
|  | 36"Ø CONC. FENDER PILE | | |
|  | NEW 30"Ø FIBERGLASS REINFORCED
PLASTIC PIPE (FRP.) JACKET | | |

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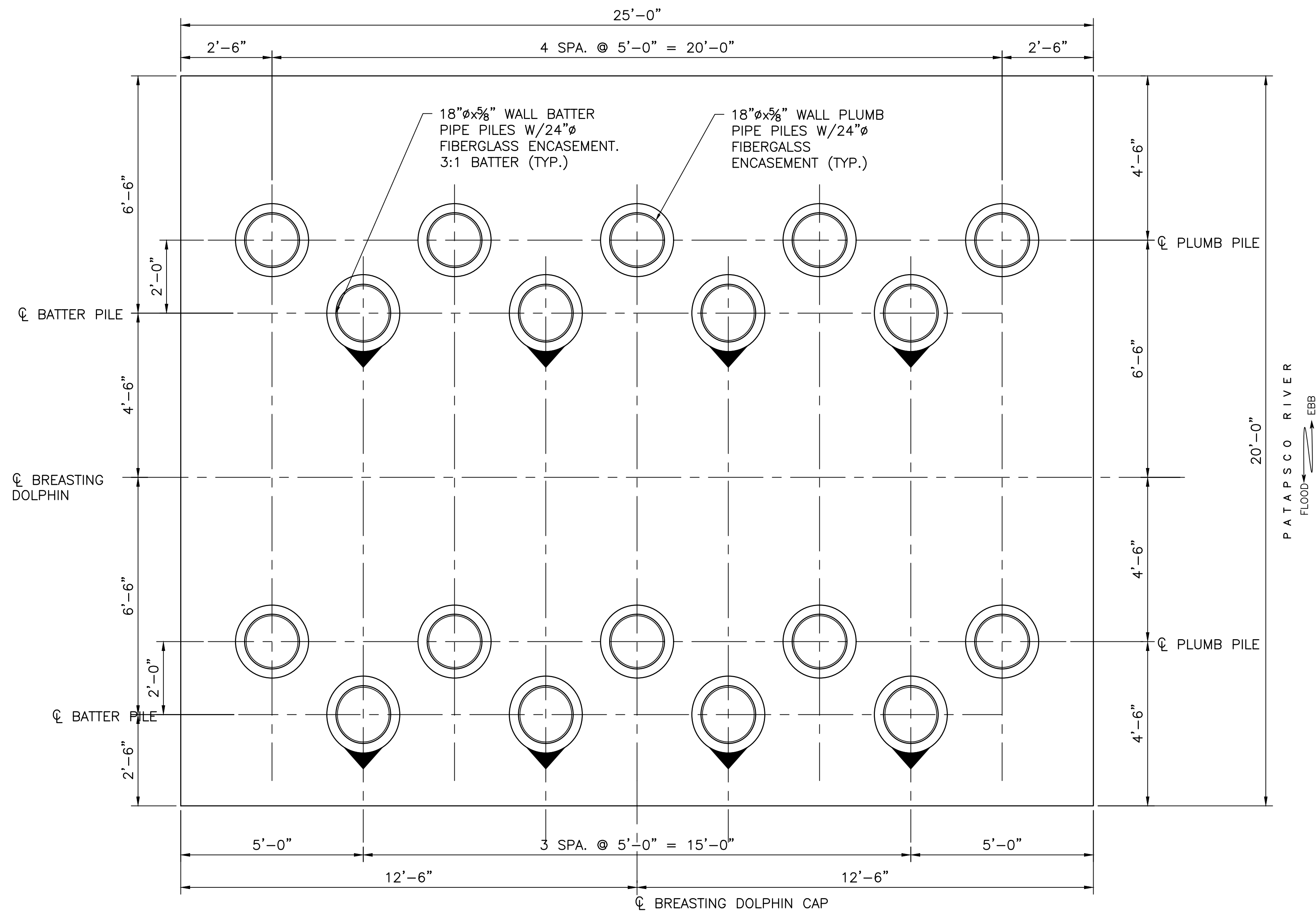
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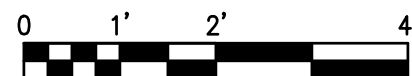
PROPOSED PILE REPAIR PLAN 3

SCALE:	DESIGN BY: MKP
AS NOTED	APPROVED BY: MGV
DATE: 02/02/22	DRAWN BY: FYC
	CHECKED BY: SWM
PROJECT NO: 2020610213.000	
SHEET NUMBER	
P-3	
SHT. NO.: SHT 1 OF TOT SH	
DRAWING FILE NO: P-1 EXIS... .DWG	REV. NO: 1



NOTE: PLAN SHOWN AT BOTTOM OF
CAP (EL. +1'-8"($\pm$))

DOLPHIN PILE PLAN



NOTES:

- FOR GENERAL NOTES, SEE DWGS. GN-1 & GN-2.
- NOTE THAT PILE PLAN IS SHOWN AT THE BOTTOM OF THE CAP ELEVATION OF +1'-8".
- THE NEW DOLPHIN HAS BEEN LOCATED TO MINIMIZE THE POTENTIAL FOR INTERFERENCE FROM ABANDONED TIMBER PILE FIELDS, HOWEVER, SOME ADJUSTMENT MAY BE NECESSARY. THE CONTRACTOR'S FRAMEWORK MUST BE FLEXIBLE ENOUGH TO ALLOW ADJUSTMENTS OF PILE LOCATIONS UP TO 5 FEET IN ANY DIRECTION.

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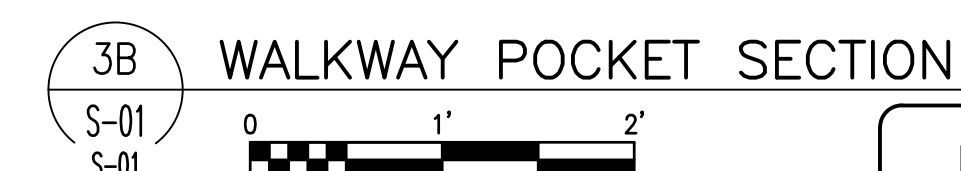
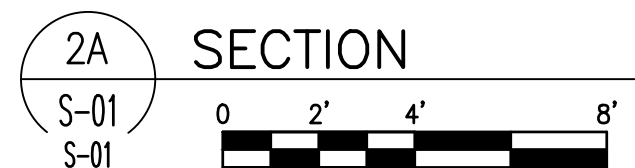
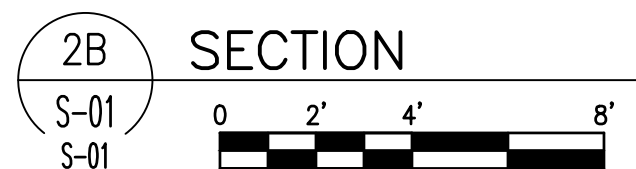
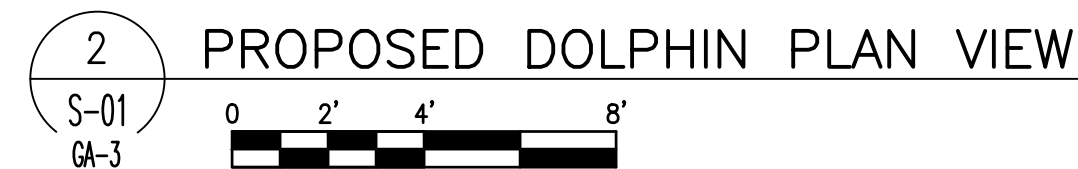
GOLD BOND BUILDING PRODUCTS, LLC
GBBP BALTIMORE NEW DOLPHIN

PROPOSED PILE PLAN

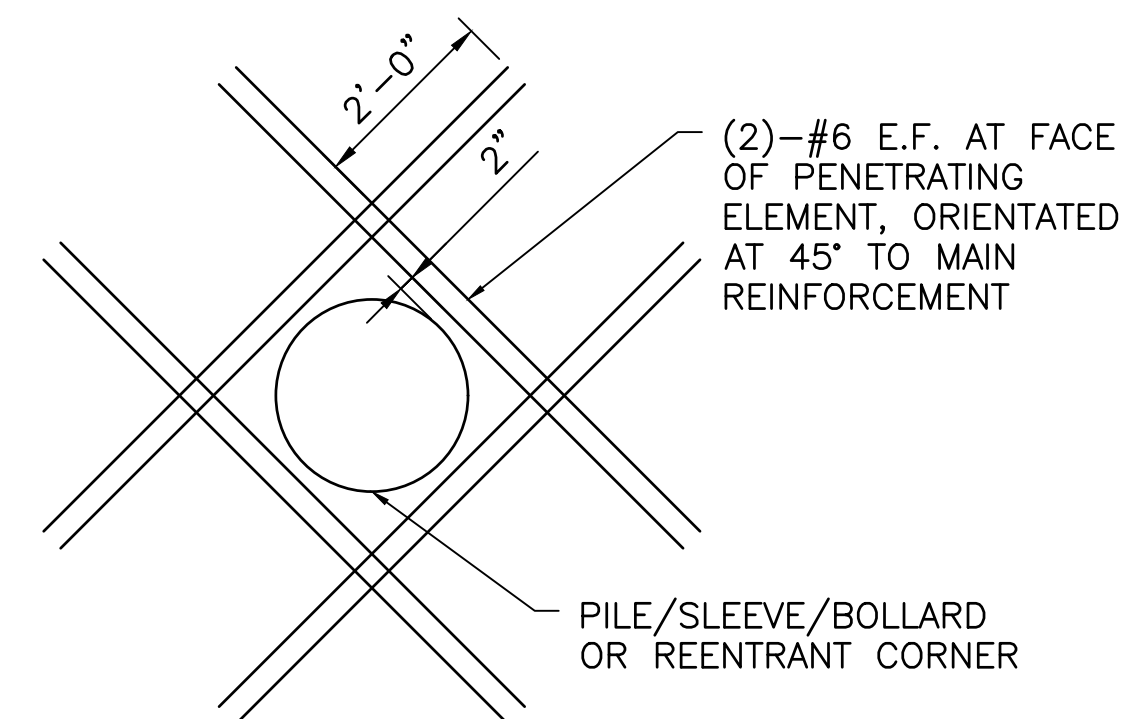
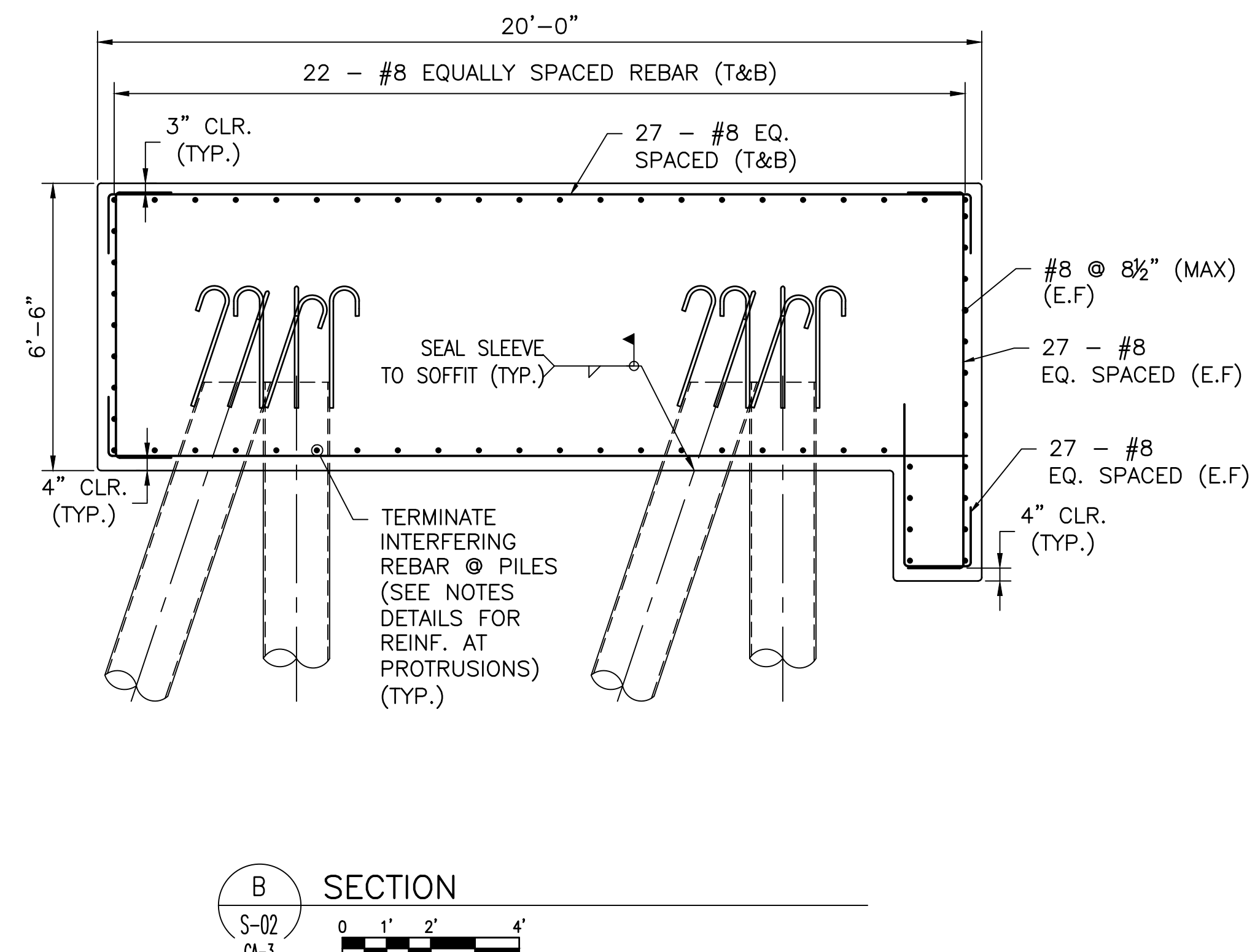
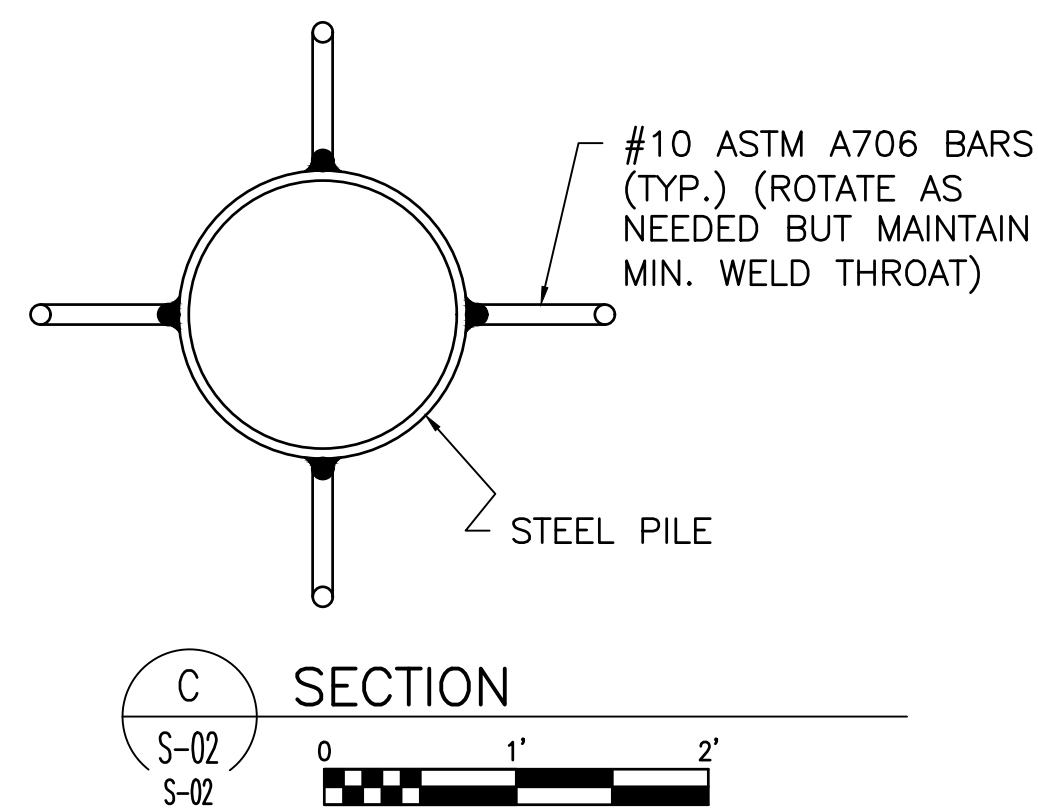
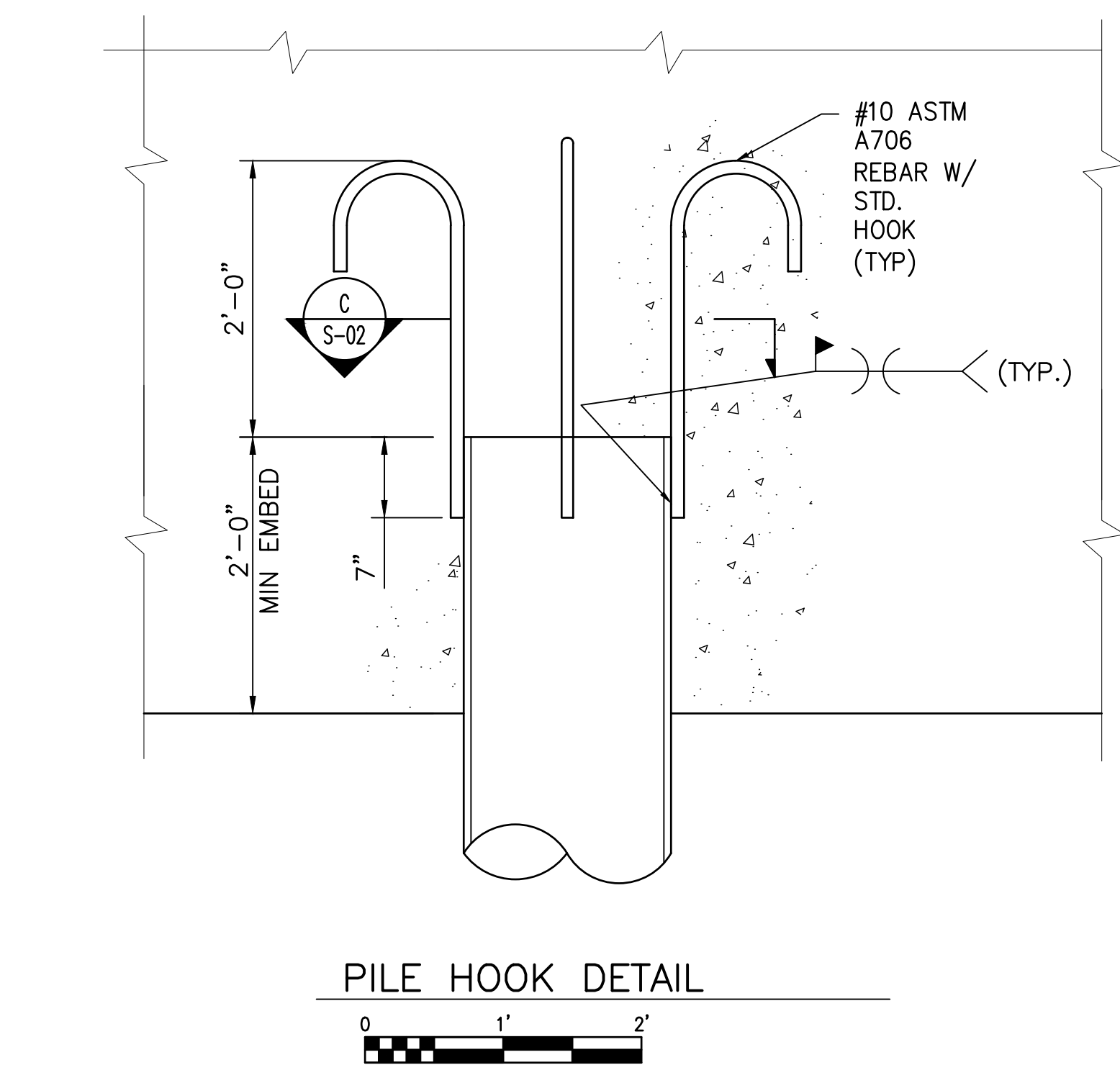
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DATE:	02/02/22	APPROVED BY:	MGW
PROJECT NO:	2020610213.000	DRAWN BY:	FJC
SHEET NUMBER:	P-4	CHECKED BY:	SNM
SHT NO.:	SHT 1	OF TOT SH	
DRAWING FILE NO.:	P-4 PROP...	REV NO.:	1

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TYPICAL SUPPLEMENTAL REINFORCEMENT
PLAN AT INTERFERENCE
N.T.S.

NOTES:

1. FOR GENERAL NOTES, SEE DWGS. GN-1 & GN-2.
2. BOLLARD ANCHORAGE NOT SHOWN FOR CLARITY. INSTALL BOLLARDS USING MANUFACTURER'S RECOMMENDED PROCEDURES, METHODS, AND MATERIALS. PILE HOOKS MAY NEED TO BE ROTATED IN PLAN TO AVOID INTERFERENCE WITH BOLLARD ANCHORS.
3. BOTTOM MAT REINFORCEMENT THAT WOULD INTERFERE WITH PILES SHALL BE DETAILED WITH STANDARD HOOKS OR TAILS FOR TERMINATION AT PILE LOCATIONS.
4. THE CONTRACTOR HAS THE OPTION OF POURING AN INITIAL LIFT FOR EASE OF FORMING; NOTE THAT AN ADDITIONAL BOTTOM MAT OF REINFORCEMENT WOULD THEN BE REQUIRED IN THE NEXT LIFT AND THAT DOWELS MAY BE REQUIRED BETWEEN LIFTS. THE CONTRACTOR SHALL SUBMIT FORMWORK DETAILS ALONG WITH A POURING PLAN FOR REVIEW.

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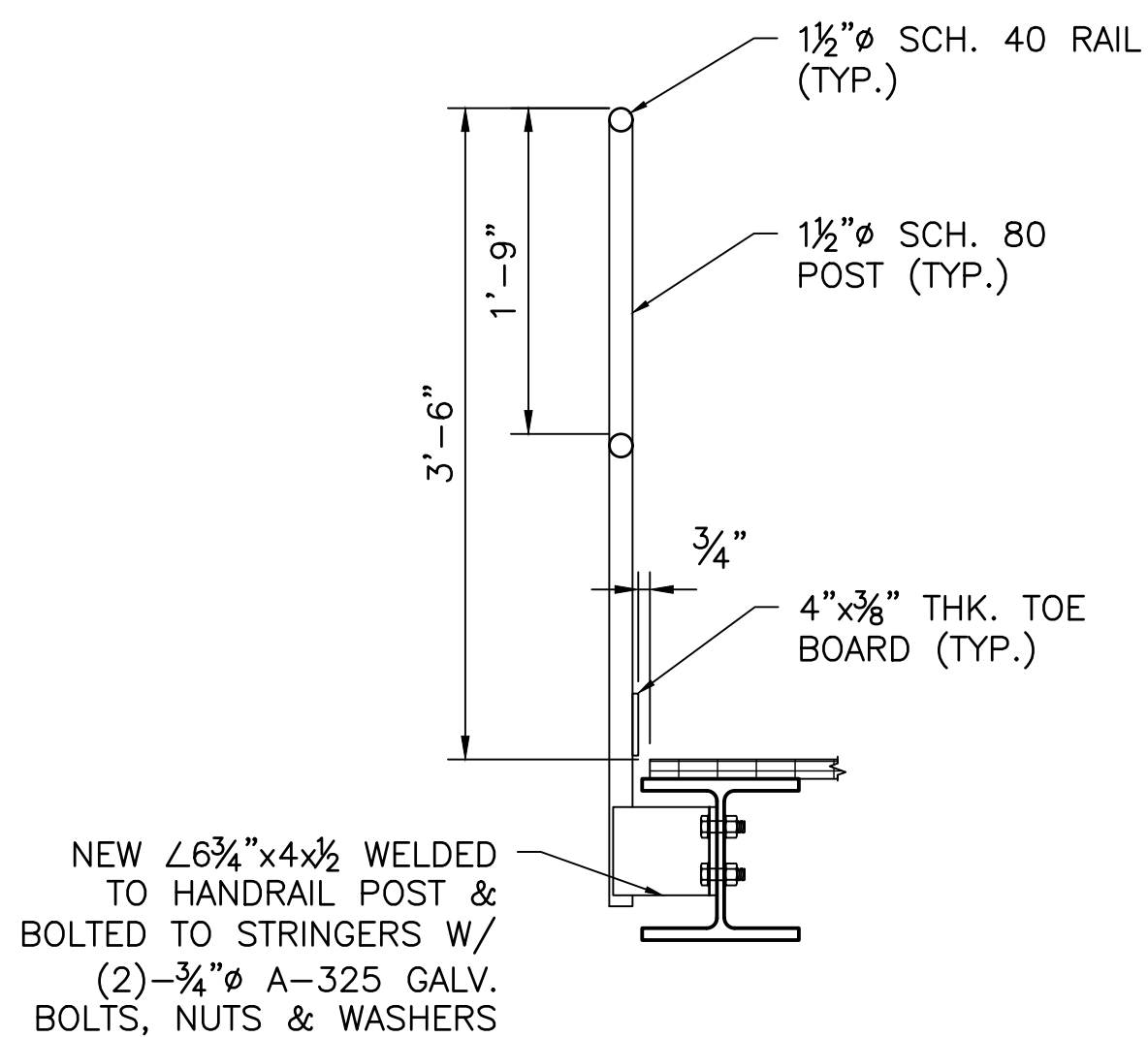
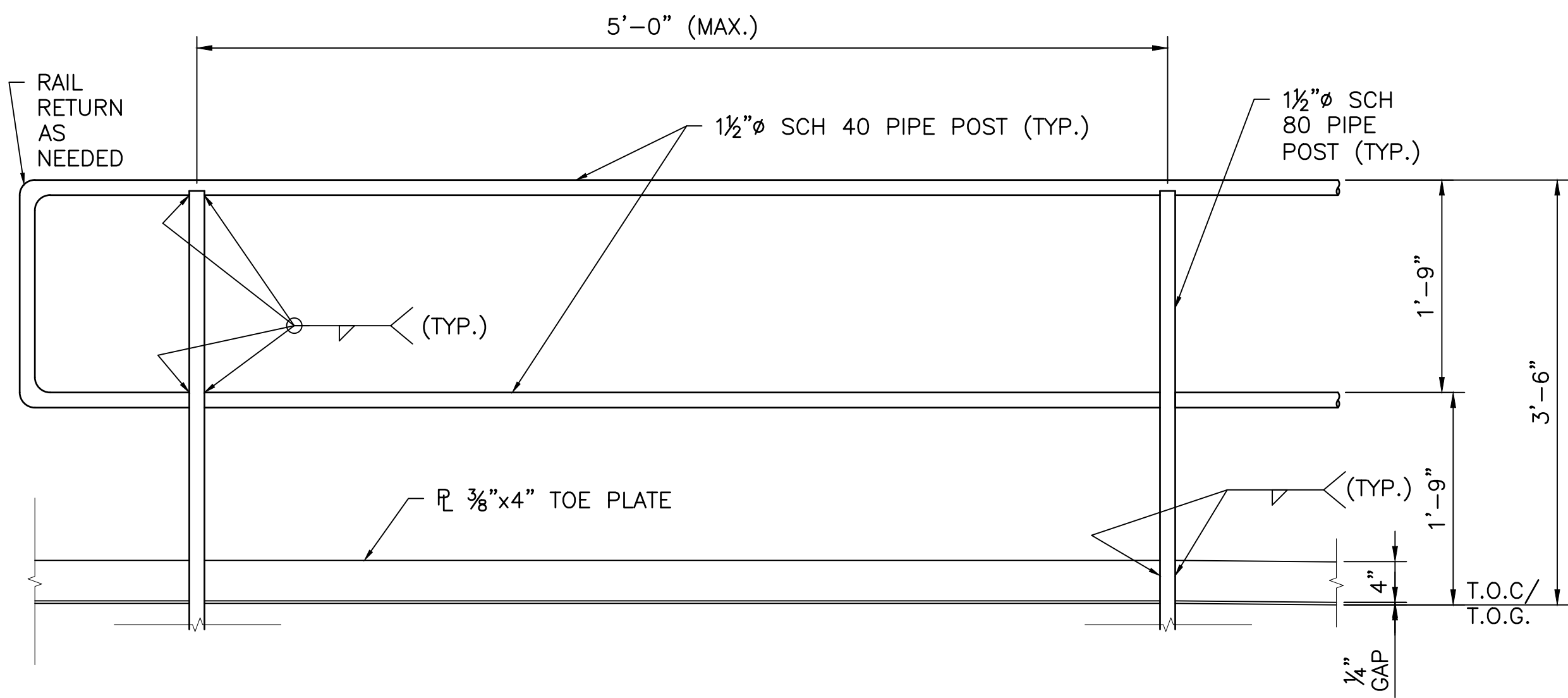
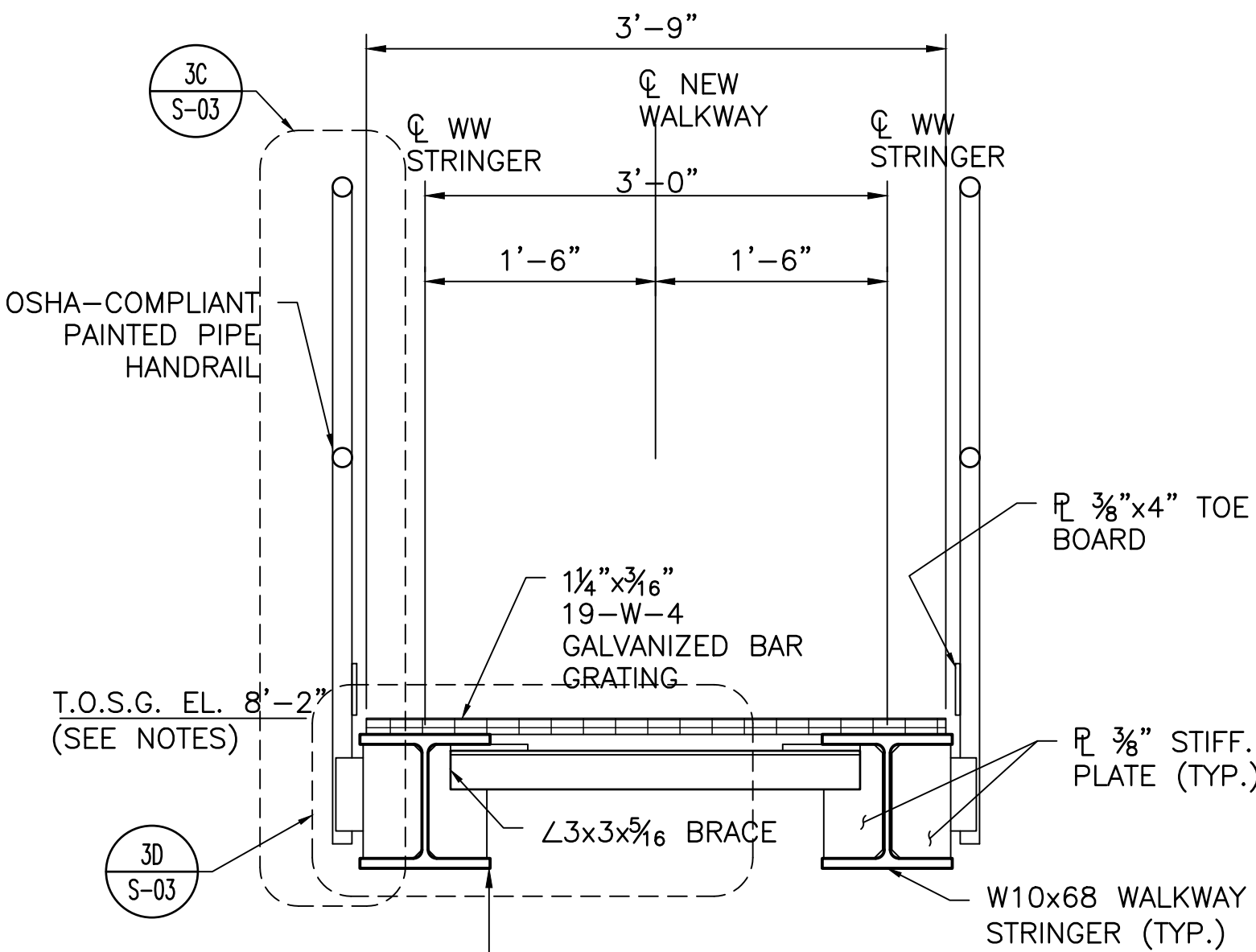
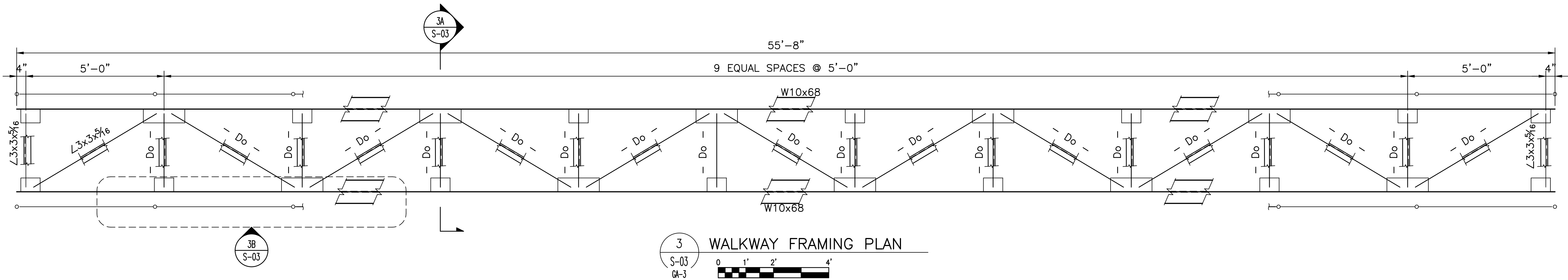
BALTIMORE, MD |

LC

GBBP BALTIMORE NEW DOLPHIN

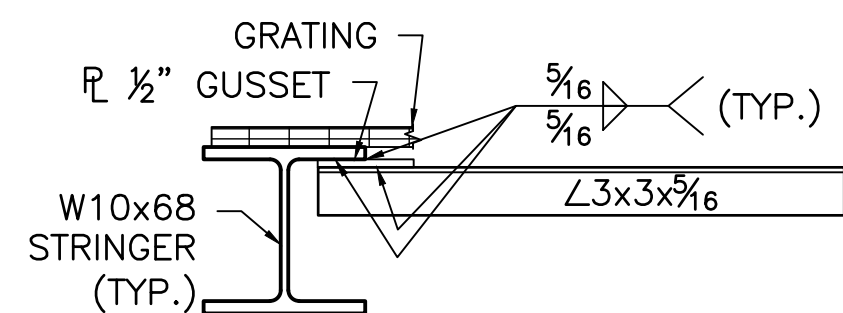
BREASTING DOLPHIN REINFORCEMENT DETAILS

SCALE: AS NOTED	DESIGN BY: MKP
	APPROVED BY: MGN
DATE: 02/02/22	DRAWN BY: FYC
	CHECKED BY: SVM
PROJECT NO.: 2020610213.000	
SHEET NUMBER S-02	
SHT. NO.: SHT 1 OF TOT SH	
DRAWING FILE NO.: S-02 BRE... .DWG	REV. NO.: 1



3B
S-03
S-03
HANDRAIL ELEVATION VIEW

3C
S-03
S-03
TYPICAL WALKWAY SECTION



3D
S-03
S-03
TYPICAL WALKWAY SECTION

NOTES:
1. FOR GENERAL NOTES, SEE GN-1 & GN-2.

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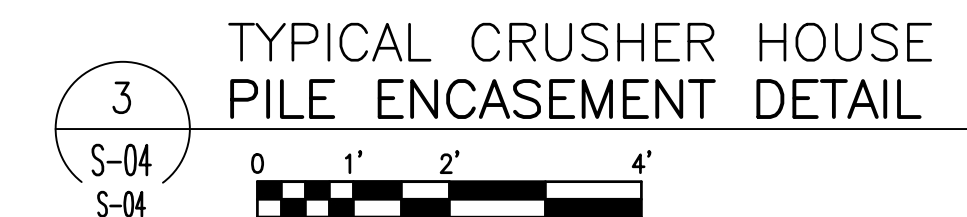
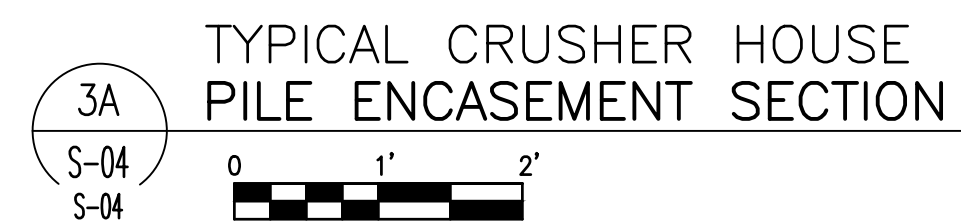
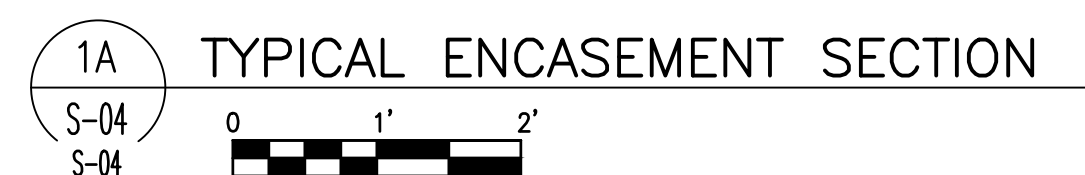
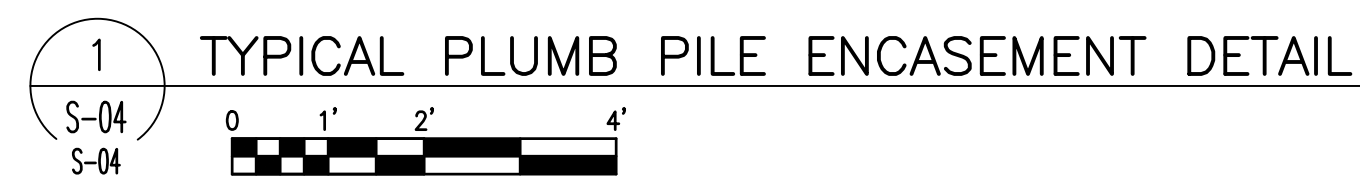
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GOLD BOND BUILDING PRODUCTS, LLC	BALTIMORE, MD
GBBP BALTIMORE NEW DOLPHIN	
WALKWAY FRAMING PLAN, SECTIONS AND DETAILS	
SCALE: AS NOTED	DESIGN BY: MPK
DATE: 02/02/22	APPROVED BY: MGW
PROJECT NO: 2020610213.000	DRAWN BY: FVC
SHEET NUMBER: S-03	CHECKED BY: BMM
SHT NO: SHT 1 OF TOT SH	
DRAWING FILE NO: S-01.dwg...	REV NO: 1



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